

# CAIS STANDARD MANUAL

# SYSTEM NO. 16 BRIDGES



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CAS PROJECT
CAIS MANUAL

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# MEMORANDUM FOR DTIC-OCP

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FROM: AL/EQ (STINFO)

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SUBJECT: Transmision of Technical Documents

1. As per telephone conversation with Andrew Poulis, EQ/TIC, the attached CAIS CTDS manuals are forwarded for accession, cataloging, and microconversions. Please forward the accession numbers to:

Andrew Poulis AL/EQ/TIC 139 Barnes Drive. Suite 2 Tyndall AFB, FL 32403-5323

- 2. The Distribution statement should read as follows: Approved for Public Release: Distribution Unlimited.
- 3. If you have questions about these documents, please contact Andrew Poulis at DSN 523-6285.

LARRY L. TESTERMAN
Scientific and Technical

Information Program Manager

Atchs: Manuals

# **TABLE OF CONTENTS**

		PAGE
ABSTRAC <sup>-</sup>	Т	
	SYSTEM 16 BRIDGES	
INSPECTO	R'S GUIDE	. 1
1.	General	. 1
11.	General Inspection	
111.	Inspector Qualifications	2
IV.	Inspection Unit (IU)	
V.	Unit Costs	. 3
VI.	Standard Safety Requirements	з
VII.	Standard Tools	. 3
VIII.	Special Tools and Equipment Requirements	3
IX.	Level II Inspection Method Keys	3
X.	Level III Inspection Method Keys	4
XI.	Replacement Costs	. 4
XII.	Appendices	. 4
SYSTEM T	REE	. 5
	SUBSYSTEM 16.01 ABUTMENTS	
DESCRIPTI	ON	. 6
Speci	al Tool and Equipment Requirements	. 6
	al Safety Requirements	
Comp	onent List	. 6
Relate	ed Subsystems	. 6
Stand	lard Inspection Procedure	. 7
	onents	
	ences	
Guide	Sheet Control Number	. 19
Level	II Inspection Method Guide Sheets	. 20
Level	III Inspection Method Guide Sheets	. 22

	PAGE
SUBSYSTEM 16.02 - PILES	
DESCRIPTION	34
Special Tool and Equipment Requirements Special Safety Requirements Component List Related Subsystems Standard Inspection Procedure Components References Guide Sheet Control Number Level II Inspection Method Guide Sheets Level III Inspection Method Guide Sheets	34 34 35 35 44 45
SUBSYSTEM 16.03 - PIERS	
DESCRIPTION	61
Special Tool and Equipment Requirements Special Safety Requirements Component List Related Subsystems Standard Inspection Procedure Components References Guide Sheet Control Number Level II Inspection Method Guide Sheets Level III Inspection Method Guide Sheets	61 61 62 62 79 80 81
SUBSYSTEM 16.04 SUPERSTRUCTURE	
DESCRIPTION	99
Special Tool and Equipment Requirements Special Safety Requirements Component List Related Subsystems Standard Inspection Procedure Components References Guide Sheet Control Number Level II Inspection Method Guide Sheet Level III Inspection Method Guide Sheets	100 100 100 101 115 116

F	PAGE
SUBSYSTEM 16.05 EXPANSION JOINTS	
DESCRIPTION	135
Special Tool and Equipment Requirements Special Safety Requirements Component List Related Subsystems Standard Inspection Procedure Components References Guide Sheet Control Number Level II Inspection Method Guide Sheets Level III Inspection Method Guide Sheets	135 136 136 136 137 160 161 162 164
SUBSYSTEM 16.06 BRIDGE RAILING AND PARAPETS	
DESCRIPTION	179
Special Tool and Equipment Requirements Special Safety Requirements Component List Related Subsystems Standard Inspection Procedure Components References Guide Sheet Control Number Level III Inspection Guide Sheets	179 179 179 179 179 180 188 189
SUBSYSTEM 16.07 DRAINAGE	
DESCRIPTION	193
Special Tool and Equipment Requirements Special Safety Requirements Component List Related Subsystems Standard Inspection Procedure Components References Guide Sheet Control Number	193 193 193 193 193 194 200 201

Issued 4/28/95 Rev. 0

	PAG	ìΕ
	APPENDICES	
APPENDIX A	- ABBREVIATIONS A-	-1
APPENDIX B	- GLOSSARY B-	-1
APPENDIX C	- LIFE CYCLE	-1

#### **ABSTRACT**

#### **GENERAL ORGANIZATION**

At this installation the list of facilities to be surveyed, including infrastructure, will be addressed on the basis of 32 unique systems that form the CAIS Engineering Deficiency Standards and Inspection Methods document. Each system deals with a specific technical aspect of the facility to be surveyed. Within each system a further breakdown is made to subsystems, each having a related list of components. Detailed observations of the listed defects are provided so as to allow the entry of observed quantification data. A DOD CAIS manual is provided for each of the 32 systems with an internal organization as outlined below:

# **INSPECTOR'S GUIDE**

# I. General

- A. Level I Inspection Method Description
- B. Level II Inspection Method Description
- C. Level III Inspection Method Description

#### II. General Inspection

- A. Process. This section describes the process of the inspection activity.
- B. Location. This section describes the procedure for locating the inspection units in the facility or infrastructure on this installation.

## III. Inspector Qualifications

This section notes the minimum qualifications for the person or persons performing the survey.

## IV. Inspection Unit

This section describes how the IU (Inspection Unit) is determined for the particular component being surveyed.

#### V. Unit Costs

This section notes the nature of repair costs for this system.

# VI. Standard Safety Requirements

This section lists safety procedures and equipment required to implement a safe environment for the conduct of this survey.

#### VII. Standard Tools

This section lists a set of standard tools required for the general conduct of this survey.

## VIII. Special Tools and Equipment Requirements

This section refers to special tools or equipment requirements endemic to the nature of the system being surveyed.

#### IX. Level II Inspection Method Keys

This section explains the use of keys as they relate to Level II Guide Sheets.

# X. Level III Inspection Method Keys

This section explains the use of keys as they relate to Level III Guide Sheets.

#### XI. Replacement Cost

This section describes the nature and location of replacement cost data.

# XII. Appendices

Appendix A. Provides a listing and definition of all abbreviations used both in the Standards and in the data base.

Appendix B. Provides a glossary of terms with their definitions as used in the Standard.

Appendix C. This section contains a listing of the average life cycle durations for each assembly\* in the Standard.

\* Assembly is a term describing the level at which replacement rather than repair occurs. This can be at the subsystem or component designation, depending on the system being surveyed.

#### **SYSTEM TREE**

The System Tree is a graphical representation of the Work Breakdown Structure, showing system, subsystem and component relationships for the Bridges System.

#### **INSPECTION METHODS**

#### Description

Describes the nature of what is to be condition surveyed.

#### Special Tool and Equipment Requirements

Lists any special tools required for this specific subsystem.

## **Special Safety Requirements**

This section outlines any special safety measures or equipment required for this specific subsystem so as to maintain a safe environment and process in the conduct of the condition survey.

#### Component List

All components to be surveyed under this subsystem are listed here.

# Related Subsystems

All other subsystems that have a survey relationship to this subsystem are listed here to help coordinate a complete and thorough condition assessment survey.

#### Standard Inspection Procedure

This statement indicates the various levels of survey effort required for this subsystem.

## Components

The previously listed components of this subsystem are described with a survey procedure recommended on a component by component basis. For each component there is a listing of defects with each defect broken down into observations describing the nature and severity of the defective condition observed. The surveyor enters a quantification value for each defect/observation encountered in the field CAIS device (DCD) to record the result of his survey.

# References

This page lists the reference sources from which the foregoing subsystem data was developed.

### **Guide Sheet Control Number**

This section lists the key numbers that tie the written Level II and Level III guide sheets to specific components in this subsystem.

# Level II and Level III Inspection Method Guide Sheets

This section contains the detailed descriptions of the Level II and III survey and inspection procedures for this subsystem.

Issued 4/28/95 Rev. 0 16 BRIDGES - Page - vii

#### **INSPECTOR'S GUIDE**

#### I. GENERAL

# A. Level I Inspection Method

The purpose of the Level I Inspection Method is to detect observable defects in a bridge structure. A bridge is an active structure, subject to repeated loading, erosion, corrosion, and deterioration by wind, water, ice and temperature. A well documented inspection will observe these deteriorations and identify needed repairs that, if carried out, can prevent structural failure and provide full life cycle usage of the bridge.

The Level I Inspection Method for a bridge structure consists of an inspection of the readily accessible parts of the bridge as described in the Work Breakdown Structure (WBS). The Level I Inspection Method is a walk-by inspection while taking measurements. The standard inspection is designed to be performed by two inspector.

Depending on the bridge type, a boat may be required to observe certain components above the waterline. The level I inspection method is essentially a general inspection "swim-by" overview. The inspection can detect obvious major damage or deterioration due to collision, corrosion, or biological growth and attack. The "swim-by" inspection relies primarily on and / or tactile observations (depending on the water clarity to make condition assessments). Minimum of 3 persons are required for a Level I "swim-by" inspection.

The observations recorded by the inspector during a Level I inspection are designed to create a historical data base for the continued design life of the structure, to highlight particular items which require inspection by a qualified engineer experienced in the design and construction of bridge structure (whichever is appropriate), and to devise maintenance and repair strategy.

# B. Level II Inspection Method

A Level II Inspection Method is performed to obtain additional information or measurements concerning a defect observed during the course of the Level I inspection process. In all instances, the Level II Inspection Method is additional work performed by the inspector during performance of the Level I inspection.

# C. Level III Inspection Method

Level III inspections should be conducted by an engineer or team of engineers experienced in the design and construction of bridge structure and should include a thorough systematic evaluation of the condition triggering the Level III inspection and an assessment of the safety and stability of the bridge structure.

Level III inspections should be performed when triggered by conditions observed during a Level I or Level II inspection or on a regularly scheduled basis, whichever occurs first (see Facility Manager Guide). In addition, Level III inspections should also be performed where inspection of appurtenant work components require difficult access

Issued 4/28/95 Rev. 0 16 BRIDGES - Page - 1

methods to be used or when work to be performed by others is required prior to gaining access for the inspection.

Depending on the assessment of the potential impact of observed conditions on the safety or stability of the bridge, advanced test and inspection methods may be required as part of the Level III inspection to determine the cause and/or extent of an observed defect.

Level III underwater inspection must be accomplished by a certified diver(s).

# II. GENERAL INSPECTION

# A. Process

The Level I inspection shall be carried out for each component listed in the WBS for bridges. Potential defects have been identified along with relevant observations, allowing the inspector to prepare a record of observable conditions at the bridge site. The inspector will identify the defect, record the observation, and take measurements as necessary to record the quantity or extent of the defect. No attempt will be made to officially assess the safety of the bridge structure during this inspection. The observation of certain defects will automatically trigger a Level III inspection to be performed in order to confirm the severity of the observed defect and to assess the safety of the bridge.

Level I and Level II inspections are to be performed with one or more personnel accompanying the inspector at the bridge site as specifically recommended in these documents.

#### B. Location

Level I and II inspection will be located by the inspector through a discrete entry into the data Collection Device. The "IU" or component location will be derived from facility supplied segment numbering lists, maps or other I.D. numbering systems. For Building associated "IU's" and components the Facility shall furnish plans annotated with room number schedules. In the case of non-room associated components, plans will be orientated with the top of each sheet being the north direction, so as to allow direction location and description. In the case where no maps, or plans are available the inspector shall enter a brief (65 character), description of location.

# III. INSPECTOR QUALIFICATIONS

The personnel performing standard inspections of a bridge should be a journeyman and have a minimum of 5 years experience in inspection of bridge structure. Inspectors shall be trained in the CAS system and its operation and shall be CAS certified.

# IV. INSPECTION UNIT (IU)

The IU is normally defined at the subsystem level. Components have been identified for the bridge subsystem which are to be inspected where applicable. In order to create a historical data base for the structure, there is also opportunity at the component level to identify the location and order of magnitude of the observed defect. Occasionally the IU will occur at the component level. In those cases it will be noted in the subsystem description.

#### V. UNIT COSTS

The unit costs that are applied to the quantities recorded for each observation are contained within the Site CAIS as repair cost.

# VI. STANDARD SAFETY REQUIREMENTS

Prior to inspection of the bridge, the authority (Facility Manager) having jurisdiction shall be notified to secure proper access, safety briefings, and personal safety items. See Master Safety Plan for additional requirements.

#### VII. STANDARD TOOLS

Ice pick

Employee Identification Card - to be worn or carried during all inspections
Data Collection Device (DCD)
Battery pack for DCD
100 ft tape measure
Measuring wheel
Camera: 35 mm
Calipers
Marking paint
Binoculars
Flashlight
Flat blade screwdriver
Wire brush
Hammer

# VIII. SPECIAL TOOLS AND EQUIPMENT REQUIREMENTS

At the component level, no special tools and equipment are required for the Level I inspection of the associated components. Level III Guide Sheets will address additional tools and equipment requirements that are specific to that particular method. Inspectors should review these sections in order to determine any special tool requirements for components they are to inspect.

#### IX. LEVEL II INSPECTION METHOD KEYS

Certain defect observations or the designated inspection of certain components will trigger a Level II inspection. The Facility Manager will be able to identify defects where a Level II inspection is flagged. The Level II key at the observation level will refer to a specific guide sheet.

Issued 4/28/95 Rev. 0 16 BRIDGES - Page - 3

### X. LEVEL III INSPECTION METHOD KEYS

Certain observations will trigger a Level III inspection. The Level III key at the observation level will refer to a specific guide sheet. The Facility Manager will be able to identify deficiencies where a Level III inspection is flagged. These guide sheets, will identify the Level III inspection may refer the Facility Manager to a more sophisticated inspection method.

#### XI. REPLACEMENT COST

A replacement cost for each subsystem will be contained within the cost estimating system in the Site CAIS. Remedial measure costs to correct observed defects will be estimated by the engineer(s) subsequent to the results of a Level III inspection.

#### XII. APPENDICES

# Appendix A - Abbreviations

A summary and definition of all abbreviations used in this system are contained in Appendix A which is located at the end of Bridges.

# Appendix B - Glossary

A glossary of terms used in this system are contained in Appendix B which is located at the end of Bridges.

# Appendix C - Life Cycles

A listing of the average life cycle durations for each assembly\* in the Standard.

## Note - Facility Manager's Guide

The following are included in the Facility Manager's Guide:

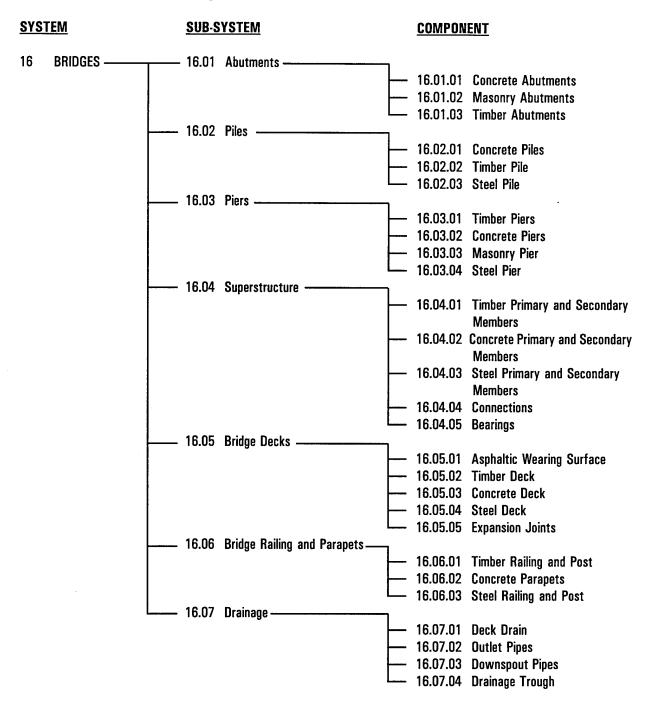
A table showing the required manhours to perform the standard inspection for this facility listed by Cat Code (three digit).

A listing of all Level III inspections with their estimated cost and time to perform. This list will include frequency of inspection for time driven Level III's.

\* Assembly is a term describing the level at which replacement rather than repair occurs. This can be at the subsystem or component designation, depending on the system being surveyed.

Issued 4/28/95 Rev. 0 16 BRIDGES - Page - 4

Figure 16-A. WORK BREAKDOWN STRUCTURE



Issued 4/28/95 Rev. 0

#### DESCRIPTION

An abutment is a substructure unit located at the end of a bridge. Its function is to provide end support for the bridge and to retain the approach embankment. Abutments are classified according to their location with respect to the approach embankment. The most common abutment types are; full height or closed types and open or spill-through type. The primary material used in the construction are plain cement concrete, reinforced concrete, stone masonry, timber or a combination of these materials. Plain Concrete and stone masonry abutments are usually gravity structures while reinforced concrete abutments are mostly cantilever or counterfort types.

# SPECIAL TOOL AND EQUIPMENT REQUIREMENTS

The following list of special tools and equipment, beyond the requirements listed in the Standard Tool Section, may be required to perform the inspection of this subsystem.

- 1. Boat
- 2. Related safety equipment

# **SPECIAL SAFETY REQUIREMENTS**

Level I and Level II inspections are performed by walking the perimeter of the bridge abutment and observing defects from ground level thus passing traffic is a hazard. The inspection must be performed with prior approval of the Facility Manager, who will notify the necessary authorities so as to provide traffic safety measures and access. It is suggested that the inspector wear an orange safety vest.

Depending on the bridge type a boat may be required to observe certain components. The inspectors are required to take all necessary safety measures. Refer to the Master Safety Plan for guidance and compliance.

#### COMPONENT LIST

- ◆ 16.01.01 CONCRETE ABUTMENT
- ♦ 16.01.02 MASONRY ABUTMENT
- ◆ 16.01.03 TIMBER ABUTMENT

#### **RELATED SUBSYSTEMS**

Due to the related nature of the elements requiring inspection, the following should be reviewed for concurrent inspection activities.

13.02	RETAINING WALLS
19.01	ROADWAYS
20.01	RAILROAD
21.00	WATERFRONT (all subsystems)
29.00	SITE ELECTRICAL (all subsystems)

Issued 4/28/95 Rev. 0 16 BRIDGES - Page - 6

## STANDARD INSPECTION PROCEDURE

The Level I inspection shall be carried out for each component listed. The inspector will identify any physical defects existing in each component; will note the level of severity of each defect type; and measure the quantity of each defect. All observed defect/observation data will be entered by the inspector into the Data Collection Device.

# **COMPONENTS**

## **◆ 16.01.01 CONCRETE ABUTMENT**

The primary abutment elements are: bridge seat, back wall, breast wall and wingwalls. The bridge seat provide a bearing area which supports the bridge superstructure. The backwall is the topmost portion of an abutment. Its primary function is to retain the soil and keep it from sliding onto the bridge seat and provide support for the approach slab. The breast wall supports the superstructure live and dead loads and retains the approach fill. The breast wall also consolidates and transmits the loads to the footings or piles. Wingwalls are the walls on each side of an abutment which enclose the approach.

Potential defects which may be observed include drainage, scour or erosion, vertical, lateral and rotational movement of the abutment due to instability of the soil and deterioration of the concrete surfaces.

Defect:	UOM	LEVEL II KEY	LEVEL III KEY
* Drains or Weep Holes:			
Observation:			
a. Drains and weep hole clogged.	EA		
* * * {Severity L}			
<ul> <li>b. Signs of water stain on the face of abutment around cracks.</li> </ul>	LF		
***{Severity M}			
c. Visible signs of water seeping through cracks or joints in the abutment.	LF		
***{Severity H}			
Defect:			

# \* Erosion or Scouring at Abutment or Wingwalls:

Observation:		
a. Voids less than 2".	SF	
* * * {Severity L}		
b. Voids greater than 2" less than 6".	SF	
* * * {Severity H}		
c. Undermining of abutment, with voids greater than 6".	SF	1
* * * {Severity H}		

Issued 4/28/95 Rev. 0 16 BRIDGES - Page - 7

# **COMPONENTS (Continued)**

# ◆ 16.01.01 CONCRETE ABUTMENT (Continued)

Defect:		UOM	LEVEL II	LEVEL III KEY
* H	lorizontal Cracks:			
	Observation:			
	<ul><li>a. Hairline cracks less than 1/16" wide.</li><li>***{Severity L}</li></ul>	LF		
	<ul> <li>b. Medium crack greater than 1/16" less than 1/8" wide. Staining of concrete surface with signs of efflorescence deposit and spalling of cracks.</li> <li>***{Severity M}</li> <li>c. Wide cracks greater than 1/8" wide. Staining of concrete surface</li> </ul>	LF LF	1 .	1
Defect:	with efflorescence deposit, spalling of cracks, and reinforcing bars exposed.  ***{Severity H}			
* D	iagonal Cracks - Breastwall/wingwalls: Observation:			
	<ul><li>a. Hairline cracks less than 1/16" wide.</li><li>***{Severity L}</li></ul>	LF		
	b. Medium crack greater than 1/16" less than 1/8" wide. Staining of concrete surface with signs of	LF	1	

\*\*\*{Severity M}
c. Wide cracks greater than 1/8"
wide. Staining of concrete surface
with efflorescence deposit, spalling
of cracks, and reinforcing bars exposed.

efflorescence deposit and spalling

\*\*\*{Severity H}

of cracks.

1

LF

<b>COMPONENTS</b>	(Continu	ed)

•	16.01.01	CONCRETE	ABUTMENT	(Continued)
---	----------	----------	----------	-------------

Defect:		UOM	LEVEL II KEY	LEVEL III KEY
	cal Cracks:			
Ot	oservation:			•
a. **	Hairline cracks less than 1/16" wide  *{Severity L}	LF		
b.		LF	1 .	
c.	*{Severity M} Wide cracks greater than 1/8" wide. Staining of concrete surface with efflorescence deposit, spalling of cracks, and reinforcing bars exposed. *{Severity H}	LF		1

# **Defect:**

# \* Joints - Backwall and Wingwalls:

Observation:

a. Joint separation or movement less than 1/8".	LF
* * * {Severity L}	
b. Joint separation or movement greater	LF
than 1/8" less than 1/4".	
* * * {Severity M}	
c. Joint separation or movement greater	LF

than 1/4".
\*\*\*{Severity H}

# **Defect:**

# \* Scaling:

Observation:

a. Loss of surface mortar greater than 1/4" deep SF and less than 1/2" deep with exposed aggregate.

\*\*\*{Severity L}

b. Loss of surface mortar greater than SF 1/2 " deep, less than 1" deep. Coarse aggregates are clearly exposed.

\*\*\*{Severity M}

c. Loss of coarse aggregate particles, SF as well as mortar surrounding coarse aggregates; depth of the loss greater than 1" deep, reinforcing bars exposed.

\*\*\*{Severity H}

1

1

# **COMPONENTS (Continued)**

# ◆ 16.01.01 CONCRETE ABUTMENT (Continued)

Defect:	UOM	LEVEL II KEY	LEVEL III KEY
* Rotational Movement:  Some abutments are constructed with battered or slope front face.  Observation:			
a. Abutment walls rotated inward or outward.  * * * {Severity H}	LF		1
<ul><li>b. Wing walls rotated inward or outward.</li><li>***{Severity H}</li></ul>	LF		1
Defect:			
* Bearing Seat - Breast Wall:  (Critical where beam bears directly on the Abutment wall.)  Observation:			
<ul><li>a. Light spalling and chipping of concrete.</li><li>***{Severity L}</li></ul>	SF		
<ul><li>b. Dirt and debris accumulated on bearing seat.</li><li>***{Severity L}</li></ul>	SF		
<ul><li>c. Spalling, cracking of concrete at edge of seat.</li><li>*** {Severity M}</li></ul>	SF		
<ul> <li>d. Severe spalling and cracking with crushing of concrete and exposed reinforcing bars.</li> <li>***{Severity H}</li> </ul>	SF		1
Defect:			
* Vehicular Damage:			
Observation: a. Member out-of-alignment.	SF		

SF

\*\*\*{Severity H}

\*\*\*{Severity H}

b. Member cracked, crushed or missing.

# **COMPONENTS (Continued)**

# ◆ 16.01.01 CONCRETE ABUTMENT (Continued)

Defect:		UOM	LEVEL II KEY	LEVEL III KEY
* Spall	ling:			
Ol	bservation:			
a.	Depression less than 1" deep and less than 6" in diameter.	SF		
* *	* *(Severity L)			
b.		SF	1 .	
* *	* *{Severity M}			
c.	Depression greater than 1" deep and greater than 6" in diameter with corroded re-bars.	SF		1
* :	* * {Severity H}			
Defect:				

# 

# \* Popout:

Observation:

a.	Conical shape holes less than 1/2"	SF
	diameter.	
* *	*{Severity L}	
b.	Conical shape Hole greater than 1/2" less	SF
	than 2 1/2 inch diameter.	
* *	*{Severity M}	
c.	Conical shape hole greater than 2-1/2"	SF

in diameter. \* \* \* {Severity M}

# **COMPONENTS (Continued)**

#### ♦ 16.01.02 MASONRY ABUTMENT

The primary elements of a masonry abutment are bridge seat, stem wall or breastwall, wingwalls and footing. The bridge seat provides a bearing area which supports the bridge superstructure. The stem or breast wall supports the superstructure live and dead loads and retains the approach fill. The stem or breast wall also consolidates and transmits the loads to the footing. Wingwalls are the walls on each side of an abutment which enclose the approach.

Potential defects which may be observed include drainage, scour or erosion, vertical, lateral and rotational movement of the abutment due to instability of the soil and deterioration of the masonry surfaces.

Defect:		UOM	LEVEL II KEY	LEVEL III KEY
* Drains or Weep Holes:				
Observation:				
a. Drains and wee	p hole clogged.	EA		
* * * {Severity L}				
b. Signs of water	stain on the face of	LF		
	vegetation growth at joints.	,		
* * * {Severity M}				
• • •	water seeping through crac	ks LF		
or joints in the				
* * * {Severity H}				
Defect:				
			•	

# \* Erosion or Scouring at Abutment or Wingwalls:

Observation:		
a. Voids less than 2".	SF	
* * * {Severity L}		
b. Voids greater than 2", less than 6".	SF	
* * * {Severity H}		
c. Undermining of abutment, with voids	SF	2
greater than 6".	,	
* * * {Severity H}		

# **COMPONENTS (Continued)**

# ◆ 16.01.02 MASONRY ABUTMENT (Continued)

Defect:	UOM	LEVEL II KEY	LEVEL III KEY
* Mortar Joint:			
Observation:			
a. Mortar joint cracked with no voids masonry stone sound.	SF		
<pre>***{Severity L} b. Mortar joint deteriorated with voids,</pre>	SF		
<ul> <li>c. Mortar joint totally deteriorated, masonry stone missing.</li> <li>***{Severity H}</li> </ul>	SF		2
Defect:			
* Masonry Stone Deterioration:			
Observation:			
<ul> <li>Masonry stone, minor spalling and hairline cracks.</li> </ul>	SF		
* * * {Severity L}			
<ul> <li>b. Masonry stone, spalling with cracks</li> <li>and chipping, stone loose.</li> <li>***{Severity M}</li> </ul>	SF		
c. Masonry stone, spalling with large cracks and chipping, section loss greater than 15%.  ***{Severity H}	SF		2
Defect:			
* Joints - Abutment and Wingwalls: Observation:			
<ul><li>a. Joint separation or movement less than 1/2".</li><li>*** {Severity L}</li></ul>	LF		
b. Joint separation or movement greater 1/2" less than 1".  ***{Severity M}	LF		
c. Joint separation or movement greater than 1' ***{Severity H}	'. LF		2

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# **COMPONENTS (Continued)**

# ◆ 16.01.02 MASONRY ABUTMENT (Continued)

Defect:	UOM	LEVEL II KEY	LEVEL III KEY
* Vehicular Damage:			
Observation:  a. Member out-of-alignment.  * * * {Severity H}	SF		2
<ul><li>b. Member cracked, crushed or missing.</li><li>***{Severity H}</li></ul>	SF		2
Defect:			
* Rotational Movement: Observation:			
<ul><li>a. Abutment walls rotated inward or outward.</li><li>***{Severity H}</li></ul>	LF		2
<ul><li>b. Wing walls rotated inward or outward.</li><li>***{Severity H}</li></ul>	LF		2
Defect:			
* Bearing Seat:  (Critical where beam bears directly on the Abutment wall.)  Observation:			
<ul><li>a. Light spalling and chipping of masonry.</li><li>***{Severity L}</li></ul>	SF		
<ul><li>b. Dirt and debris accumulated on bearing seat.</li><li>***{Severity L}</li></ul>	SF		
c. Spalling, cracking of masonry at edge of seat.  ***{Severity M}	SF		
<ul><li>d. Severe spalling and cracking with crushing of masonry.</li><li>***{Severity H}</li></ul>	SF		2

**Defect:** 

LEVEL III

**KEY** 

**LEVEL II** 

KEY

**UOM** 

# **16.01 ABUTMENTS**

#### **COMPONENTS** (Continued)

#### ◆ 16.01.03 TIMBER ABUTMENT

The primary elements of a timber abutment are bridge seat, stem wall or breastwall, wingwalls, footing and piles. The bridge seat provides a bearing area which supports the bridge superstructure. The stem or breast wall supports the superstructure live and dead loads and retains the approach fill. The stem or breast wall also consolidates and transmits the loads to the footing. Piles are often used to provide lateral support for stem or breast wall and support for the superstructure. Wingwalls are the walls on each side of an abutment which enclose the approach.

The potential defects which may be observed include scour or erosion, vertical, lateral and rotational movement due to soil instability and deterioration to timber members.

		• • • • • • • • • • • • • • • • • • • •		,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,
* Bearing \$	Seat:			
	vation:			
	Moist and stained, surface solid. Severity L}	SF		
b. D	oirt and debris accumulated on bearing eat.	SF		
* * * {S	Severity L}			
С	Noist and stained, surface soft, slight rushing.	SF	2	
-	Severity M}			
	rea soft and crumbly and seriously eteriorated.	SF		3
* * * {S	everity H}			
Defect:				
* Erosion o	or Scouring at Abutment or Wingwalls:			
	oids less than 2". everity L}	SF		
b. V	oids greater than 2" less than 6". everity H}	SF		
c. `U	Indermining of abutment, with voids reater than 6".	SF		3
* * * {S	everity H}			

# **COMPONENTS (Continued)**

# ◆ 16.01.03 TIMBER ABUTMENT (Continued):

Defect:	UOM	LEVEL II KEY	LEVEL III KEY
* Decay - Breast/Wingwalls:  Decay from rot/fungus is most likely to occur at connections, splices, support points or around bolt holes. This may be due either to the tendency of such area to collect and retain moisture, or to bolts holes or cuts being made in the surface after the preservative treatment has been applied.			
Observation:  a. Moist and stain or discolored area, signs of fungi, surface is solid.  * * * {Severity L}	SF		
b. Surface spongy, member may shown signs of crushing.  * * * {Severity M}	SF	2	
c. Brown and white - discolored area, member may show section loss and crushing.  * * * {Severity H}	SF		3
Defect:			
* Parasites - Breast/Wingwalls and Pile Lagging: (Termites, carpenter ants, powder post beetles) Observation:			
<ul> <li>a. Pinholes with dark stain area around the holes.</li> <li>*** {Severity L}</li> </ul>	SF		
<ul> <li>b. Holes less than 1/8" dia., surface sag, and sawdust observed.</li> </ul>	SF	2	
<pre>***(Severity M} c. Holes greater than 1/8" dia., surface</pre>	SF		3

# **COMPONENTS (Continued)**

# ◆ 16.01.03 TIMBER ABUTMENT (Continued)

Defect:	UOM	KEY	LEVEL III KEY
* Rotational Movement - Breast Wall/Wingwalls: Observation:			
<ul><li>a. Breast wall rotated inward or outward.</li><li>***{Severity H}</li></ul>	EA		3
<ul><li>b. Wing wall rotated inward or outward.</li><li>***{Severity H}</li></ul>	EA		3
Defect:			
* Weathering: Observation:			
<ul> <li>a. Surface of wood is rough and corrugated and member may be warped.</li> <li>***{Severity L}</li> </ul>	SF		
<ul> <li>b. Surface of wood is rough and corrugated with cracks partially through the wood member, may have minor section loss.         Member may be warped.         ***{Severity M}     </li> </ul>	SF	2	
c.Large cracks extends deeply or completely through the wood.  ***{Severity H}	SF		3
<ul><li>d. Wood is crumbly and seriously deteriorated.</li><li>***{Severity H}</li></ul>	SF		3
Defect:			
* Vehicular Damage: Observation:			
a. Member out-of-alignment.  * * * {Severity H}	SF		3
<ul><li>b. Member cracked, crushed or missing.</li><li>***{Severity H}</li></ul>	SF		3

# **REFERENCES**

- 1. U.S. Department of transportation, Federal Highway Administration, Bureau of Public roads (DOT) Bridge Inspector's Training Manual, 1990 Edition.
- Bridge inspection and Rehabilitation, Parsons Brinckerhoff Edited by L.G. Silano, PE, 1993
- 3. Inspection of Bridges and Trestles NAVFAC MO-126, October 1991
- 4. ACI Manual of Concrete Practice 1989, Part 1 Materials and General Properties of Concrete
- AASHTO Manual for Maintenance Inspection of Bridges, American Association of State Highway and Transportation Officials
- 6. Micro Bridger (Version 1.0), U.S. Army Construction Engineering Research Laboratory

LEVEL II KEY	GUIDE SHEET CONTROL NUMBER
1 2	GS-II 16.01.01-1 GS-II 16.01.02-2
LEVEL III KEY	GUIDE SHEET CONTROL NUMBER
1	GS-III 16.01.01-1

# LEVEL II GUIDE SHEET - KEY NO. 1

COMPONENT:

**CONCRETE ABUTMENT** 

**CONTROL NUMBER:** 

GS-II 16.01.01-1

# **Application**

This applies to the investigation of concrete bridge abutment deterioration due to spalling from delamination. The results of the Level II inspection can be used to trigger a level III inspection or necessary repair.

# **Special Safety Requirements**

No special Safety Requirements, beyond the requirements listed in the Safety Section, are to be observed in the performance of the level II.

# **Inspection Actions**

- 1. Clean loose concrete from area to be inspected.
- 2. Measure the affected area.
- 3. Tap the affected area with a hammer to determine extent of unsound or hollow concrete.

#### **Recommended Inspection Frequency**

When triggered by Level I inspection and where Level II is utilized as the standard inspection procedure.

## References

- 1. U.S. Department of transportation, Federal Highway Administration, Bureau of Public Roads (DOT) Bridge Inspector's Training Manual, 1990 Edition
- 2. Bridge inspection and Rehabilitation, Parsons Brinckerhoff Edited by L.G. Silano, PE, 1993
- 3. Inspection of Bridges and Trestles NAVFAC MO-126, October 1991
- ACI Manual of Concrete Practice 1989, Part 1 Materials and General Properties of Concrete
- AASHTO Manual for Maintenance Inspection of Bridges, American Association of State Highway and Transportation Officials
- 6. Micro Bridger (Version 1.0), U.S. Army Construction Engineering Research Laboratory

#### LEVEL II GUIDE SHEET - KEY NO. 2

COMPONENT:

**TIMBER ABUTMENT** 

**CONTROL NUMBER:** 

GS-II 16.01.02-2

## Application

This applies to the investigation of possible interior and exterior deterioration of timber abutment due to decay (rot), or parasites.

The results of the Level II inspection can be used to trigger a level III inspection or necessary repair.

# **Special Safety Requirements**

No special Safety Requirements, beyond the requirements listed in the Safety Section, are to be observed in the performance of the Level II.

# **Inspection Actions**

- 1. Clean affected area.
- 2. Measure affected area.
- 3. Tap with hammer to determine extend of hollow or sound material.
- 4. Probe with ice pick.

# **Recommended inspection Frequency**

When triggered by Level I inspection and where Level II is utilized as the standard inspection procedure.

#### References

- 1. U.S. Department of transportation, Federal Highway Administration, Bureau of Public Roads (DOT) Bridge Inspector's Training Manual, 1990 Edition
- 2. Bridge inspection and Rehabilitation, Parsons Brinckerhoff Edited by L.G. Silano, PE, 1993
- 3. Inspection of Bridges and Trestles NAVFAC MO-126, October 1991
- 4. AASHTO Manual for Maintenance Inspection of Bridges, American Association of State Highway and Transportation Officials
- 5. Micro Bridger (Version 1.0), U.S. Army Construction Engineering Research Laboratory

DOD CAS MANUAL 16 Bridges

# LEVEL III INSPECTION METHOD GUIDE SHEET

#### LEVEL III GUIDE SHEET - KEY NO. 1

COMPONENT:

**CONCRETE ABUTMENTS** 

**CONTROL NUMBER:** 

GS-III 16.01.01-1

#### Application

This guide has been prepared to identify the purpose of a Level III inspection and its more sophisticated test and inspection methods which may be appropriate to determine the cause and/or extent of defects recorded in Level I or Level II defect observations at a concrete abutment.

Whereas the purpose of the Level I inspection was to record the observable defects at the abutments, this Level III inspection is performed to provide a thorough systematic evaluation of the observed defect and to make an assessment of it's effects, if left unchecked, on the safety, durability and stability of the abutment and it's appurtenant works.

The Level III inspection should be performed when prompted by the results of a Level I or II inspection. The inspection should be performed by an engineer or multidisciplined team of engineers experienced in the design and construction of bridges.

The results of the Level III inspection will be used to develop maintenance or remedial measure work strategy that will correct the existing deficiency conditions or to require continued monitoring of existing deficiency conditions at the concrete abutments.

In general, appropriate advanced inspection methods will be identified, recommended, and performed by or under the supervision of the inspection engineer personnel as part of the Level III test and inspection method. Advanced inspection methods will be assigned only after the assessment of defect conditions observed during a Level I or II inspection.

## **Special Safety Requirements**

Passing traffic is a hazard. Level III inspection and testing must be performed with the prior approval of the Facility Manager who will notify the authorities to provide safety measures and safe access.

Issued 4/28/95 Rev. 0 16 BRIDGES - Page - 22

# LEVEL III GUIDE SHEET - KEY NO. 1 (Continued)

COMPONENT:

**CONCRETE ABUTMENTS** 

**CONTROL NUMBER:** 

GS-III 16.01.01-1

# **Inspection Actions**

- Prior to making a field inspection of the observed defect, review all past records concerning the abutments and the defective component if available. These records may include pre-construction investigation records, design criteria and analysis records, available construction records, previous periodic maintenance inspection records, water level record and photographs taken during initial construction and subsequent inspections.
- 2. Perform inspection of the pertinent components where observed defects that triggered a Level III inspection are listed.
- Make an assessment of the importance of individual defects observed for a given component at the bridge site. Indicate priorities for any required maintenance, or remedial measure work.
- 4. Identify whether particular observed defects need additional or continued observation.
- 5. Assess the stability and safety of the abutment.
- Prepare final cost estimate for advanced inspection methods required to determine the cause and extent of the observed defect.
- 7. Prepare cost estimate for required maintenance or remedial repair measures, as applicable.

#### LEVEL III GUIDE SHEET - KEY NO. 1 (Continued)

COMPONENT:

**CONCRETE ABUTMENTS** 

**CONTROL NUMBER:** 

GS-III 16.01.01-1

Level III advanced inspection methods may be required for several Level I and Level II defect conditions observed at a bridge site. Level III advanced test or inspection methods and associated observed defects for concrete abutments include, but are not limited to the following:

#### 

1. Infrared Thermography and Ground probing radar

concrete deterioration, cracks and spalling

2. Concrete coring

concrete deterioration

 Laboratory tests on concrete (core, strength tests, abrasion, absorption,sulfate soundness, unit weight

concrete deterioration

4. Ultrasonic test

cracks and voids in concrete

5. Half-cell test

corrosion to reinforcement steel

6. Soil borings

soil instability and settlement

7. Laboratory tests on soil samples (strength tests, moisture content, consolidation tests, etc.)

soil instability

8. Underwater Inspection

erosion, scouring and undermining

9. Survey measurement

abutment movement

# **Special Instructions**

Review as-built and design drawings of structure.

# **Special Tools & Equipment Requirements**

Surveying equipment
Navigable boat with related safety equipment
Industry required testing equipment needed to perform the advanced investigation
method chosen

# LEVEL III GUIDE SHEET - KEY NO. 1 (Continued)

COMPONENT:

**CONCRETE ABUTMENTS** 

**CONTROL NUMBER:** 

GS-III 16.01.01-1

# Recommended Inspection Frequency

As triggered by Level I and II defect/observations or every 3 years.

# References

- 1. U.S. Department of transportation, Federal Highway Administration, Bureau of Public roads (DOT) Bridge Inspector's Training Manual, 1990 Edition
- 2. Bridge inspection and Rehabilitation, Parsons Brinckerhoff Edited by L.G. Silano, PE, 1993
- 3. Inspection of Bridges and Trestles NAVFAC MO-126, October 1991
- 4. ACI Manual of Concrete Practice 1989, Part 1 Materials and General Properties of Concrete
- 5. AASHTO Manual for Maintenance Inspection of Bridges, American Association of State Highway and Transportation Officials
- 6. Micro Bridger (Version 1.0), U.S. Army Construction Engineering Research Laboratory

DOD CAS MANUAL 16 Bridges

#### LEVEL III INSPECTION METHOD GUIDE SHEET

#### **LEVEL III GUIDE SHEET - KEY NO. 2**

**COMPONENT:** MASONRY ABUTMENTS **CONTROL NUMBER:** GS-III 16.01.02-2

# **Application**

This guide has been prepared to identify the purpose of a Level III inspection and its more sophisticated test and inspection methods which may be appropriate to determine the cause and/or extent of defects recorded in Level I or Level II defect observations at a masonry abutment.

Whereas the purpose of the Level I inspection was to record the observable defects at the abutments, this Level III inspection is performed to provide a thorough systematic evaluation of the observed defect and to make an assessment of it's effects, if left unchecked, on the safety, durability and stability of the abutment and it's appurtenant works.

The Level III inspection should be performed when prompted by the results of a Level I or II inspection. The inspection should be performed by an engineer or multidisciplined team of engineers experienced in the design and construction of bridges.

The results of the Level III inspection will be used to develop maintenance or remedial measure work strategy that will correct the existing deficiency conditions or to require continued monitoring of existing deficiency conditions at the abutments.

In general, appropriate advanced inspection methods will be identified, recommended, and performed by or under the supervision of the inspection engineer personnel as part of the Level III test and inspection method. Advanced inspection methods will be assigned only after the assessment of defect conditions observed during a Level I or II inspection.

## **Special Safety Requirements**

Passing traffic is a hazard. Level III inspection and testing must be performed with the prior approval of the Facility Manager who will notify the authorities to provide safety measures and safe access.

#### LEVEL III GUIDE SHEET - KEY NO. 2 (Continued)

**COMPONENT:** MASONRY ABUTMENTS **CONTROL NUMBER:** GS-III 16.01.02-2

## Inspection Action

- Prior to making a field inspection of the observed defect, review all past records concerning the abutments and the defective component if available. These records may include pre-construction investigation records, design criteria and analysis records, available construction records, previous periodic maintenance inspection records, water level records and photographs taken during initial construction and subsequent inspections.
- 2. Perform inspection of the pertinent components where observed defects that triggered a Level III inspection are listed.
- Make an assessment of the importance of individual defects observed for a given component at the bridge site. Indicate priorities for any required maintenance, or remedial measure work.
- 4. Identify whether particular observed defects need additional or continued observation.
- 5. Assess the stability and safety of the abutment.
- 6. Prepare final cost estimate for advanced inspection methods required to determine the cause and extent of the observed defect.
- 7. Prepare cost estimate for required maintenance or remedial repair measures, as applicable.

Level III advanced inspection methods may be required for several Level I and Level II defect conditions observed at a bridge site. Level III advanced test or inspection methods and associated observed defects for abutments include, but are not limited to the following:

Issued 4/28/95 Rev. 0 16 BRIDGES - Page - 27

# LEVEL III GUIDE SHEET - KEY NO. 2 (Continued)

COMPONENT:

**MASONRY ABUTMENTS** 

**CONTROL NUMBER:** 

GS-III 16.01.02-2

#### 

1. Infrared Thermography and ground probing radar

masonry deterioration, cracking and spalling

2. Ultrasonic test

cracks and voids in masonry

3. soil borings

soil instability, erosion and settlement-

4. laboratory tests on soil samples (strength tests, moisture content, consolidation tests, etc.)

soil instability

5. Underwater Inspection

erosion, scouring and undermining

6. Survey measurement

abutment movement

#### **Special Instructions**

Review as-built and design drawings of structure.

#### **Special Tools & Equipment Requirements**

Equipment designated in Level I inspections
Surveying equipment
Navigable boat with related safety equipment
Industry required testing equipment needed to perform the advanced investigation method chosen

### **Recommended Inspection Frequency**

As triggered by Level I and II defect/Observations or every 3 years.

#### LEVEL III GUIDE SHEET - KEY NO. 2 (Continued)

COMPONENT:

MASONRY ABUTMENTS

**CONTROL NUMBER:** 

GS-III 16.01.02-2

#### References

 U.S. Department of transportation, Federal Highway Administration, Bureau of Public roads (DOT) Bridge Inspector's Training Manual, 1990 Edition

- 2. Bridge inspection and Rehabilitation, Parsons Brinckerhoff Edited by L.G. Silano, PE, 1993
- 3. Inspection of Bridges and Trestles NAVFAC MO-126, October 1991
- 4. ACI Manual of Concrete Practice 1989, Part 1 Materials and General Properties of Concrete
- 5. AASHTO Manual for Maintenance Inspection of Bridges, American Association of State Highway and Transportation Officials
- 6. Micro Bridger (Version 1.0), U.S. Army Construction Engineering Research Laboratory

#### **LEVEL III GUIDE SHEET - KEY NO. 3**

COMPONENT:

**TIMBER ABUTMENTS** 

**CONTROL NUMBER:** 

GS-III 16.01.03-3

#### **Application**

This guide has been prepared to identify the purpose of a Level III inspection and its more sophisticated test and inspection methods which may be appropriate to determine the cause and/or extent of defects recorded in Level I or Level II defect observations at a timber abutment.

Whereas the purpose of the Level I inspection was to record the observable defects at the abutments, this Level III inspection is performed to provide a thorough systematic evaluation of the observed defect and to make an assessment of it's effects, if left unchecked, on the safety, durability and stability of the abutment and it's appurtenant works.

The Level III inspection should be performed when prompted by the results of a Level I or II inspection. The inspection should be performed by an engineer or multidisciplined team of engineers experienced in the design and construction of bridges.

The results of the Level III inspection will be used to develop maintenance or remedial measure work strategy that will correct the existing deficiency conditions or to require continued monitoring of existing deficiency conditions at the abutments.

In general, appropriate advanced inspection methods will be identified, recommended, and performed by or under the supervision of the inspection engineer personnel as part of the Level III test and inspection method. Advanced inspection methods will be assigned only after the assessment of defect conditions observed during a Level I or II inspection.

#### **Special Safety Requirements**

Passing traffic is a hazard. Level III inspection and testing must be performed with the prior approval of the Facility Manager who will notify the authorities to provide safety measures and safe access.

#### LEVEL III GUIDE SHEET - KEY NO. 3 (Continued)

COMPONENT:

TIMBER ABUTMENTS

CONTROL NUMBER:

GS-III 16.01.03-3

#### **Inspection Action**

- Prior to making a field inspection of the observed defect, review all past records concerning the abutments and the defective component if available. These records may include pre-construction investigation records, design criteria and analysis records, available construction records, previous periodic maintenance inspection records, water level records and photographs taken during initial construction and subsequent inspections.
- 2. Perform inspection of the pertinent components where observed defects that triggered a Level III inspection are listed.
- 3. Make an assessment of the importance of individual defects observed for a given component at the bridge site. Indicate priorities for any required maintenance, or remedial measure work.
- 4. Identify whether particular observed defects need additional or continued observation.
- 5. Assess the stability and safety of the abutment.
- 6. Prepare final cost estimate for advanced inspection methods required to determine the cause and extent of the observed defect.
- 7. Prepare cost estimate for required maintenance or remedial repair measures, as applicable.

Level III advanced inspection methods may be required for several Level I and Level II defect conditions observed at a bridge site. Level III advanced test or inspection methods and associated observed defects for abutments include, but are not limited to the following:

#### LEVEL III GUIDE SHEET - KEY NO. 3 (Continued)

**COMPONENT:** 

**TIMBER ABUTMENTS** 

**CONTROL NUMBER:** 

GS-III 16.01.03-3

#### 

1. Increment borer interior and exterior deterioration of timber due to

decay or parasites

2. Ultrasonic interior voids due decay or parasites

3. Moisture content deterioration due to decay or parasites

4. Soil borings soil instability, erosion

Laboratory tests on soil samples (strength tests, moisture content,

consolidation tests, etc.)

soil instability

6. Underwater Inspection

erosion, scouring and undermining

7. Survey measurement

abutment movement

### **Special Instructions**

Review as-built and design drawings of structure.

# **Special Tools & Equipment Requirements**

Equipment designated in Level I inspections

Survey Level and rod

Navigable boat with related safety equipment

Industry required testing equipment needed to perform the advanced investigation method chosen

### **Recommended Inspection Frequency**

As triggered by Level I and II defect/Observations or every 3 years.

# **LEVEL III GUIDE SHEET - KEY NO. 3 (Continued)**

COMPONENT:

**TIMBER ABUTMENTS** 

**CONTROL NUMBER:** 

GS-III 16.01.03-3

### References

1. U.S. Department of transportation, Federal Highway Administration, Bureau of Public roads (DOT) Bridge Inspector's Training Manual, 1990 Edition

- 2. Bridge inspection and Rehabilitation, Parsons Brinckerhoff Edited by L.G. Silano, PE, 1993
- 3. Inspection of Bridges and Trestles NAVFAC MO-126, October 1991
- 4. AASHTO Manual for Maintenance Inspection of Bridges, American Association of State Highway and Transportation Officials
- 5. Micro Bridger (Version 1.0), U.S. Army Construction Engineering Research Laboratory

#### **DESCRIPTION**

Piles are substructure elements of a bridge that transmit the loads from the superstructure and/or footing to the underlying soil or rock. Piles are generally completely buried, and, therefore, cannot be visually inspected. However piles which are exposed are used as intermediate supports for a bridge when multiple spans are required and are referred to as pile bents. Pile bents are transverse structural frames composed of piles and pile cap. Piles are constructed of reinforced concrete with (convention reinforcement), timber and steel.

#### SPECIAL TOOL AND EQUIPMENT REQUIREMENTS

The following list of special tools and equipment, beyond the requirements listed in the Standard Tool Section, may be required to perform the inspection of this subsystem.

- 1. Boat
- 2. Related safety equipment
- 3. Diving gear and all related safety equipment

### SPECIAL SAFETY REQUIREMENTS

Level I and Level II inspections are performed by walking the perimeter of the bridge pile bents and observing defects from ground level thus passing traffic is a hazard. The inspection must be performed with prior approval of the Facility Manager, who will notify the necessary authorities so as to provide traffic safety measures and access. It is suggested that the inspector wear an orange safety vest.

Depending on the bridge type, a boat may be required to observe certain components. The inspectors are required to take all necessary safety measures and refer to the Master Safety Plan for guidance and compliance. The underwater inspection must be accomplished by a certified diver, as indicated in the introduction of this book, and in the Level III key description.

#### **COMPONENT LIST**

- **♦** 16.02.01 CONCRETE PILE
- ◆ 16.02.02 TIMBER PILE
- ◆ 16.02.03 STEEL PILE

#### **RELATED SUBSYSTEMS**

Due to the related nature of the elements requiring inspection, the following should be reviewed for concurrent inspection activities.

13.02	RETAINING WALLS
19.01	ROADWAYS
20.01	RAILROAD
21.00	WATERFRONT (all subsystems)

# STANDARD INSPECTION PROCEDURE

The Level I inspection shall be carried out for each component listed. The inspector will identify any physical defects existing in each component; will note the level of severity of each defect type; and measure the quantity of each defect.

#### **COMPONENTS**

#### **♦ 16.02.01 CONCRETE PILE**

A concrete pile is a vertical or inclined structural members that are driven, jetted, jacked or cast in placed into the ground. Their purpose is to transmit vertical and lateral loads from the superstructure into or through the soil stratum. Both above and underwater inspection portion of the piles shall be inspected. Underwater inspection by diver(s) will be a Level III inspection.

Potential defects which may be observed include collision damage and deterioration of the concrete surfaces.

Defect:	aces.	иом	LEVEL II KEY	LEVEL III KEY
Splash	ete Disintegration - Inspect Waterline,  Zone and Ground Line Areas: servation:			
a.	Hollow spaces or voids present within concrete aggregate exposed concrete sound.   *{Severity L}	s SF		
b.		SF		
c.	Hollow spaces or voids present within concrete with exposed reinforcing bars. {Severity H}	SF		1
Defect:				
* Vertic	al Cracks - Piles, Pile Cap:			

# De

Observation:

a. Hairline cracks less than 1/16" wide.	LF		
* * * {Severity L}			
b. Medium crack greater than 1/16" less than	LF	1	
1/8" wide. Staining of concrete surface	•		
with signs of efflorescence deposit			
and spalling of cracks.			
* * * {Severity M}			
c. Wide cracks greater than 1/8"	LF		1
wide. Staining of concrete surface			
with efflorescence deposit and spalling			
of cracks, reinforcing bars exposed.			
* * * {Severity H}			

# **COMPONENTS (Continued)**

# ◆ 16.02.01 CONCRETE PILE (Continued)

Defect:	UOM	LEVEL II KEY	LEVEL III KEY
* Horizontal Cracks - Piles, Pile Cap:			
Observation:			
<ul> <li>a. Hairline cracks less than 1/16" wide</li> </ul>	LF		
* * * {Severity L}			
<ul> <li>b. Medium crack greater than 1/16" less than 1/8" wide. Staining of concrete surface with signs of efflorescence deposit</li> </ul>	LF	1 .	
and spalling of cracks.			
* * * {Severity M}			
c. Wide cracks greater than 1/8" wide. Staining of concrete surface with efflorescence deposit and spalling of cracks, reinforcing bars exposed.	LF		1
* * * {Severity H}			

# **Defect:**

# \* Bearing Seat - Pile Cap:

_				
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,	bse	-: I V		_

<ul> <li>a. Light spalling and chipping of concrete.</li> </ul>	SF	
* * * {Severity L}		
b. Dirt and debris accumulated on bearing	SF	
seat.		
* * * {Severity L}		
c. Spalling, cracking of concrete at edge	SF	
of seat.		
* * * {Severity M}		
d. Severe spalling and cracking with	SF	1
crushing of concrete and exposed		
reinforcing bars.		
* * * {Severity H}		

# **COMPONENTS (Continued)**

# ◆ 16.02.01 CONCRETE PILES (Continued)

Defect:	•	UOM	LEVEL II KEY	LEVEL III KEY
* Spalling:				
Observation:				
a. Depression less the than 6" in diamete	an 1" deep and less r.	SF		
* * * (Severity L)				
b. Depression greater greater than 6" in		SF	1 .	
* * * {Severity M}				
<ul><li>c. Depression greater greater than 6" in corroded re-bars.</li></ul>	•	SF		1
* * * {Severity H}				
Defect:				

SF

SF

#### C

# \* Scaling:

Observation:

- Loss of surface mortar greater than SF 1/4" deep and less than 1/2" deep with exposed aggregate.
- \*\*\*{Severity L}
- b. Loss of surface mortar greater than 1/2" deep less than 1" deep. Coarse aggregates are clearly exposed.
- \*\*\*{Severity M}
- c. Loss of coarse aggregate particles, as well as mortar surrounding coarse aggregates; depth of the loss greater than 1" deep, reinforcing bars exposed.
- \* \* \* {Severity H}

1

# **COMPONENTS (Continued)**

# ◆ 16.02.01 CONCRETE PILE (Continued)

Defect:		UOM	LEVEL II KEY	LEVEL III KEY
* Popo	out:			
0	bservation:			
a.	Conical shape holes less than 1/2" diameter.	SF		
*	**{Severity L}			
b.	•	SF		
*	* *{Severity M}			
c.	Conical shape hole greater than 2-1/2" in diameter.	SF		
*	**{Severity M}			
Defect:				
* 0 111				

# \* Collision Damage:

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a.	Member out-of-alignment.	LF	1
* * *	*{Severity H}		
b.	Member cracked, crushed or	LF	1
	missing.		

#### **COMPONENTS (Continued)**

#### **◆ 16.02.02 TIMBER PILE**

A timber pile is a vertical or inclined structural member that is driven, jacked or jetted into the ground. Their purpose is to transmit vertical and lateral loads from the superstructure into or through the soil stratum. Both above and underwater inspection portion of the piles shall be inspected. Underwater inspection by diver(s) will be a Level III inspection.

Potential defects which may be observed include collision damage and deterioration of the timber surfaces.

Defect:	UOM	LEVEL II KEY	LEVEL III KEY
* Deep Abrasions or Excessive Wear: Inspect waterline, splash zone and ground line areas. Observation:			
<ul><li>a. Diameter loss less than 10%.</li><li>***{Severity L}</li></ul>	EA		
<ul><li>b. Diameter loss less than 25%.</li><li>***{Severity M}</li></ul>	EA		
<ul><li>c. Diameter loss greater than 25%.</li><li>***{Severity H}</li></ul>	EA		2
Defect:			
* Splits - Piles, Pile Cap: Observation:			
<ul><li>a. Partial split in member.</li><li>***{Severity L}</li></ul>	LF		
<ul><li>b. Split completely through member.</li><li>***{Severity M}</li></ul>	LF		
<ul><li>c. Member split and failed.</li><li>***{Severity H}</li></ul>	LF		2

# **COMPONENTS (Continued)**

# ◆ 16.02.02 TIMBER PILE (Continued)

Defect:	UOM	LEVEL II KEY	LEVEL III KEY
* Decay: Decay from rot/fungus is most likely to occur at connections, splices, support points or around bolt holes. This may be due either to the tendency of such areas to collect and retain moisture, or to bolt holes or cuts being made in the surface after the preservative treatment has been applied.			
Observation:  a. Moist and stained or discolored area, signs of fungi, surface is solid.  * * * {Severity L}	SF		
b. Surface spongy, member may shown signs of crushing.  * * * {Severity M}	SF	2	
c. Brown and white - discolored area, member may show section loss and crushing.  ***{Severity H}	SF		2
Defect:			
* Parasites: Observation:			
<ul><li>a. Pinholes with dark stain</li><li>area around the holes.</li><li>***{Severity L}</li></ul>	SF		
b. Holes less than 1/8" dia., surface sag, and sawdust observed.  * * * (Severity M)	SF	2	
c. Holes greater than 1/8" dia., surface channels, and crushing of	SF		2

the member.
\*\*\*{Severity H}

# **COMPONENTS (Continued)**

# ◆ 16.02.02 TIMBER PILE (Continued)

Defect:	UOM	LEVEL II KEY	LEVEL III KEY
* Bearing Seat - Pile Cap:			
Observation:			
<ul><li>a. Moist and stained, surface solid.</li><li>***{Severity L}</li></ul>	SF		
b. Dirt and debris accumulated on bearing seat.	SF		
* * * {Severity L}			
<ul> <li>c. Moist and stained, surface soft, slight crushing.</li> </ul>	SF	2	
* * * {Severity M}			
<ul> <li>d. Area soft and crumbly and seriously deteriorated.</li> </ul>	SF		2
* * * {Severity H}			
Defect:			
* Connections:			
Observation:			
a. Loose fasteners.	EA		

Observation:	
a. Loose fasteners.	EΑ
* * * {Severity L}	
b. Member broken, spilt or damage.	LF
* * * {Severity H}	
c. Missing fastener or anchors.	LF
***{Severity H}	

# Defect:

# \* Collision Damage:

Observation:			
a. Member out-of-alignment.	LF	2	1
* * * {Severity H}			
b. Member cracked, crushed or	LF	2	1
missing.			
* * * {Severity H}			

# **COMPONENTS (Continued)**

#### ◆ 16.02.03 STEEL PILE

A steel pile is a vertical or inclined structural member that is driven, jacked or cast in placed into the ground. Their purpose is to transmit vertical and lateral loads from the superstructure into or through the soil stratum. Both above and underwater inspection portions of the piles shall be inspected. Underwater inspection by diver(s) will be a Level III inspection.

Potential defects which may be observed include corrosion, collision damage and deterioration of the steel surfaces.

Defect:	UOM	LEVEL II	LEVEL III KEY
* Corrosion - Piles and Pile Cap: Observation:			
<ul><li>a. Surface rust no pitting evident.</li><li>***{Severity L}</li></ul>	LF		
<ul> <li>b. Corrosion evident pitting and blistering of base material.</li> <li>***{Severity M}</li> </ul>	LF		
<ul><li>c. Corrosion evident with loss to base section.</li><li>***{Severity H}</li></ul>	LF		3
Defect:			
* Straightness or Buckling - Pile: Observation:			
<ul> <li>a. Sign of wrinkles in pile web or flanges.</li> </ul>	LF		
<pre>***{Severity M} b. Pile buckling. ***{Severity H}</pre>	LF		3
Defect:			
* Fungi Damaged or Marine Growth: Observation:			
<ul><li>a. Section loss less than 10%.</li><li>***{Severity L}</li></ul>	EA		
<ul><li>b. Section loss greater than 10%, less than 15%.</li><li>***{Severity M}</li></ul>	EA	3	
c. Section loss greater than 15%.  ***{Severity H}	EA		3

# **COMPONENTS (Continued)**

# ◆ 16.02.03 STEEL PILE (Continued)

Defect:	UOM	LEVEL II KEY	LEVEL III KEY
* Cracks - Pile Cap, Piles and Bracing:			
Observation:			
<ul> <li>Hairline or greater crack, fillet of flanges.</li> </ul>	LF		3
* * * {Severity H}			
<ul> <li>b. Hairline or greater crack, fillet of web.</li> </ul>	LF	•	3
* * * {Severity H}			
Defect:			
* Connectors or Fasteners:			
Observation:			
a. Loose bolts or fasteners.	EA		
* * * {Severity L}	- 4		
<ul><li>b. Missing fasteners or connectors.</li><li>***{Severity H}</li></ul>	EA		3
c. Crack in weld.	LF		3
* * * {Severity H}			
<pre>d. Crack in connection plate. ***{Severity H}</pre>	LF		3
Defect:			
* Collision Damage:			
Observation:			
a. Pile out of alignment.	LF		3
* * * {Severity H}			
<pre>b. Pile missing. ***{Severity H}</pre>	LF		3

#### **REFERENCES**

- U.S. Department of transportation, Federal Highway Administration, Bureau of Public roads (DOT) Bridge Inspector's Training Manual, 1990 Edition.
- 2. Bridge inspection and Rehabilitation, Parsons Brinckerhoff Edited by L.G. Silano, PE, 1993
- 3. Inspection of Bridges and Trestles NAVFAC MO-126, October 1991
- 4. ACI Manual of Concrete Practice 1989, Part 1 Materials and General Properties of Concrete
- 5. AASHTO Manual for Maintenance Inspection of Bridges, American Association of State Highway and Transportation Officials
- 6. Micro Bridger (Version 1.0), U.S. Army Construction Engineering Research Laboratory

LEVEL II KEY	GUIDE SHEET CONTROL NUMBER
1 2	GS-II 16.02.01-1 GS-II 16.02.02-2
3	GS-II 16.02.03-3
LEVEL III KEY	GUIDE SHEET CONTROL NUMBER
1	GS-III 16.02.01-1
2	GS-III 16.02.02-2
3	GS-III 16.02.03-3

#### **LEVEL II GUIDE SHEET - KEY NO. 1**

COMPONENT:

TIMBER PILES

**CONTROL NUMBER:** 

GS-II 16.02.01-1

### **Application**

This applies to the investigation of possible interior and exterior deterioration of timber piles due to decay (rot), or parasites.

The results of the Level II inspection can be used to trigger a level III inspection or necessary repair.

#### **Special Safety Requirements**

No special Safety Requirements, beyond the requirements listed in the Safety Section, are to be observed in the performance of the Level II.

#### **Inspection Actions**

- 1. Clean affected area.
- 2. Measure affected area.
- 3. Tap with hammer to determine extend of hollow or sound material.
- 4. Probe with ice pick.

### **Recommended inspection Frequency**

When triggered by level I inspection and where Level II is utilized as the standard inspection procedure.

### References

- U.S. Department of transportation, Federal Highway Administration, Bureau of Public roads (DOT) Bridge Inspector's Training Manual, 1990 Edition
- 2. Bridge inspection and Rehabilitation, Parsons Brinckerhoff Edited by L.G. Silano, PE, 1993
- 3. Inspection of Bridges and Trestles NAVFAC MO-126, October 1991
- 4. AASHTO Manual for Maintenance Inspection of Bridges, American Association of State Highway and Transportation Officials
- 5. Micro Bridger (Version 1.0), U.S. Army Construction Engineering Research Laboratory

#### LEVEL II GUIDE SHEET - KEY NO. 2

SYSTEM/COMPONENT:

**CONCRETE PILES** 

**CONTROL NUMBER:** 

GS-II 16.02.02-2

### **Application**

This applies to the investigation of concrete piles deterioration due to spalling from delamination.

The results of the Level II inspection can be used to trigger a level III inspection or necessary repair.

# **Special Safety Requirements**

No special Safety Requirements, beyond the requirements listed in the Safety Section, are to be observed in the performance of the level II.

#### **Inspection Actions**

- 1. Clean loose concrete from area to be inspected.
- Measure the affected area.
- 3. Tap the affected area with a hammer to determine extent of unsound or hollow concrete.

#### **Recommended Inspection Frequency**

When triggered by level I inspection and where Level II is utilized as the standard inspection procedure.

#### References

- U.S. Department of transportation, Federal Highway Administration, Bureau of Public roads (DOT) Bridge Inspector's Training Manual, 1990 Edition
- 2. Bridge inspection and Rehabilitation, Parsons Brinckerhoff Edited by L.G. Silano, PE, 1993
- Inspection of Bridges and Trestles NAVFAC MO-126, October 1991
- 4. ACI Manual of Concrete Practice 1989, Part 1 Materials and General Properties of Concrete
- 5. AASHTO Manual for Maintenance Inspection of Bridges, American Association of State Highway and Transportation Officials
- 6. Micro Bridger (Version 1.0), U.S. Army Construction Engineering Research Laboratory

# **LEVEL II GUIDE SHEET - KEY NO. 3**

**COMPONENT:** 

STEEL PILES

**CONTROL NUMBER:** 

GS-II 16.02.03-3

#### **Application**

This applies to the investigation of heavy fungi growth on steel piles.

The results of the Level II inspection can be used to trigger a level III inspection or necessary repair.

#### **Special Safety Requirements**

No special Safety Requirements, beyond the requirements listed in the Safety Section, are to be observed in the performance of the Level II.

#### **Inspection Actions**

- 1. Clean growth from area to be inspected.
- 2. Measure affected area, utilize calipers and ruler to determine an approximate pile section loss.

#### Recommended inspection Frequency

When triggered by Level I inspection and where Level II is utilized as the standard inspection procedure.

### References

- 1. U.S. Department of transportation, Federal Highway Administration, Bureau of Public Roads (DOT) Bridge Inspector's Training Manual, 1990 Edition
- 2. Bridge inspection and Rehabilitation, Parsons Brinckerhoff Edited by L.G. Silano, PE, 1993
- Inspection of Bridges and Trestles NAVFAC MO-126, October 1991
- 4. AASHTO Manual for Maintenance Inspection of Bridges, American Association of State Highway and Transportation Officials
- 5. Micro Bridger (Version 1.0), U.S. Army Construction Engineering Research Laboratory

#### LEVEL III GUIDE SHEET - KEY NO. 1

COMPONENT:

TIMBER PILES

**CONTROL NUMBER:** 

GS-III 16.02.01-1

#### **Application**

This guide has been prepared to identify the purpose of a Level III inspection and its more sophisticated test and inspection methods which may be appropriate to determine the cause and/or extent of defects recorded in Level I or Level II defect observations at a timber pile.

Whereas the purpose of the Level I inspection was to record the observable defects at the piles, this Level III inspection is performed to provide a thorough systematic evaluation of the observed defect and to make an assessment of it's effects, if left unchecked, on the safety, durability and stability of the piles and it's appurtenant works.

The Level III inspection should be performed when prompted by the results of a Level I or II inspection. The inspection should be performed by an engineer or multidisciplined team of engineers experienced in the design and construction of bridges.

The results of the Level III inspection will be used to develop maintenance or remedial measure work strategy that will correct the existing deficiency conditions or to require continued monitoring of existing deficiency conditions at the piles.

In general, appropriate advanced inspection methods will be identified, recommended, and performed by or under the supervision of the inspection engineer personnel as part of the Level III test and inspection method. Advanced inspection methods will be assigned only after the assessment of defect conditions observed during a Level I or II inspection.

### **Special Safety Requirements**

Passing traffic is a hazard. Level III inspection and testing must be performed with the prior approval of the Facility Manager who will notify the authorities to provide safety measures and safe access.

#### LEVEL III GUIDE SHEET - KEY NO. 1 (Continued)

COMPONENT:

**TIMBER PILES** 

**CONTROL NUMBER:** 

GS-III 16.02.01-1

#### **Inspection Actions**

Prior to making a field inspection of the observed defect, review all past records
concerning the piles and the defective component if available. These records may
include pre-construction investigation records, design criteria and analysis records,
available construction records, previous periodic maintenance inspection records,
reservoir level records, and photographs taken during initial construction and
subsequent inspections.

- 2. Perform inspection of the pertinent components where observed defects that triggered a Level III inspection are listed.
- Make an assessment of the importance of individual defects observed for a given component at the bridge site. Indicate priorities for any required maintenance, or remedial measure work.
- 4. Identify whether particular observed defects need additional or continued observation.
- 5. Assess the stability and safety of the piles.
- 6. Prepare final cost estimate for advanced inspection methods required to determine the cause and extent of the observed defect.
- 7. Prepare cost estimate for required maintenance or remedial repair measures, as applicable.

Level III advanced inspection methods may be required for several Level I and Level II defect conditions observed at a bridge site. Level III advanced test or inspection methods and associated observed defects for piles include, but are not limited to the following:

# LEVEL III GUIDE SHEET - KEY NO. 1 (Continued)

COMPONENT:

**TIMBER PILES** 

**CONTROL NUMBER:** 

GS-III 16.02.01-1

# 

1. Increment borer

interior and exterior deterioration of timber due to

decay or parasites

2. Ultrasonic

interior deterioration

3. Moisture content

deterioration due to decay or parasites

4. Soil borings

soil instability, movement and settlement

5. laboratory tests on soil samples (strength tests, moisture content, consolidation tests, etc.)

soil instability

6. Underwater inspection

erosion, scouring and undermining

7. Survey measurement

pile movement

### **Special Instructions**

Review as-built and design drawings of structure.

### **Special Tools & Equipment Requirements**

Surveying equipment

Increment borer

Moisture meter

Navigable boat with related safety equipment

Industry required testing equipment needed to perform the advanced investigation method chosen

Recommended Inspection Frequency

As triggered by a Level I and Level II defect/observation or every 3 years.

# LEVEL III GUIDE SHEET - KEY NO. 1 (Continued)

COMPONENT:

TIMBER PILES

**CONTROL NUMBER:** 

GS-III 16.02.01-1

### **References**

 U.S. Department of transportation, Federal Highway Administration, Bureau of Public roads (DOT) Bridge Inspector's Training Manual, 1990 Edition

- 2. Bridge inspection and Rehabilitation, Parsons Brinckerhoff Edited by L.G. Silano, PE, 1993
- 3. Inspection of Bridges and Trestles NAVFAC MO-126, October 1991
- 4. AASHTO Manual for Maintenance Inspection of Bridges, American Association of State Highway and Transportation Officials
- 5. Micro Bridger (Version 1.0), U.S. Army Construction Engineering Research Laboratory

### **LEVEL III GUIDE SHEET - KEY NO. 2**

COMPONENT:

**CONCRETE PILES** 

**CONTROL NUMBER:** 

GS-III 16.02.02-2

### **Application**

This guide has been prepared to identify the purpose of a Level III inspection and its more sophisticated test and inspection methods which may be appropriate to determine the cause and/or extent of defects recorded in Level I or Level II defect observations at a concrete pile.

Whereas the purpose of the Level I inspection was to record the observable defects at the piles, this Level III inspection is performed to provide a thorough systematic evaluation of the observed defect and to make an assessment of it's effects, if left unchecked, on the safety, durability and stability of the piles and it's appurtenant works.

The Level III inspection should be performed when prompted by the results of a Level I or II inspection. The inspection should be performed by an engineer or multidisciplined team of engineers experienced in the design and construction of bridges.

The results of the Level III inspection will be used to develop maintenance or remedial measure work strategy that will correct the existing deficiency conditions or to require continued monitoring of existing deficiency conditions at the piles.

In general, appropriate advanced inspection methods will be identified, recommended, and performed by or under the supervision of the inspection engineer personnel as part of the Level III test and inspection method. Advanced inspection methods will be assigned only after the assessment of defect conditions observed during a Level I or II inspection.

### **Special Safety Requirements**

Passing traffic is a hazard. Level III inspection and testing must be performed with the prior approval of the Facility Manager who will notify the authorities to provide safety measures and safe access.

# LEVEL III GUIDE SHEET - KEY NO. 2 (Continued)

COMPONENT:

**CONCRETE PILES** 

**CONTROL NUMBER:** 

GS-III 16.02.02-2

### **Inspection Action**

- Prior to making a field inspection of the observed defect, review all past records
  concerning the piles and the defective component if available. These records may
  include pre-construction investigation records, design criteria and analysis records,
  available construction records, previous periodic maintenance inspection records,
  water level records, and photographs taken during initial construction and
  subsequent inspections.
- 2. Perform inspection of the pertinent components where observed defects that triggered a Level III inspection are listed.
- Make an assessment of the importance of individual defects observed for a given component at the bridge site. Indicate priorities for any required maintenance, or remedial measure work.
- 4. Identify whether particular observed defects need additional or continued observation.
- 5. Assess the stability and safety of the piles.
- 6. Prepare final cost estimate for advanced inspection methods required to determine the cause and extent of the observed defect.
- 7. Prepare cost estimate for required maintenance or remedial repair measures, as applicable.

Level III advanced inspection methods may be required for several Level I and Level II defect conditions observed at a bridge site. Level III advanced test or inspection methods and associated observed defects for piles include, but are not limited to the following:

### LEVEL III GUIDE SHEET - KEY NO. 2 (Continued)

COMPONENT:

**CONCRETE PILES** 

**CONTROL NUMBER:** 

GS-III 16.02.02-2

# 

1. Infrared Thermography and ground probing radar

cracks and voids

2. Concrete coring

concrete deterioration

 Laboratory tests on concrete (core, strength tests, abrasion, absorption, sulfate soundness, unit weight concrete deterioration

4. Ultrasonic test

cracks and voids in concrete

5. Half-cell test

corrosion to reinforcement steel

6. Soil borings

soil instability, movement and settlement

7. Laboratory tests on soil samples (strength tests, moisture content, consolidation tests, etc.)

soil instability

8. Underwater inspection

erosion, scouring and undermining

9. Survey measurements

pile movement

# **Special Instructions**

Review as-built and design drawings of structure.

# **Special Tools & Equipment Requirements**

Surveying equipment

Navigable boat with related safety equipment.

Industry required testing equipment needed to perform the advanced investigation method chosen.

#### Recommended Inspection Frequency

Triggered by Level I and Level II defect/observations or every 3 years.

# LEVEL III GUIDE SHEET - KEY NO. 2 (Continued)

COMPONENT:

**CONCRETE PILES** 

**CONTROL NUMBER:** 

GS-III 16.02.02-2

### References

1. U.S. Department of transportation, Federal Highway Administration, Bureau of Public roads (DOT) Bridge Inspector's Training Manual, 1990 Edition

- 2. Bridge inspection and Rehabilitation, Parsons Brinckerhoff Edited by L.G. Silano, PE, 1993
- 3. Inspection of Bridges and Trestles NAVFAC MO-126, October 1991
- 4. ACI Manual of Concrete Practice 1989, Part 1 Materials and General Properties of Concrete
- 5. AASHTO Manual for Maintenance Inspection of Bridges, American Association of State Highway and Transportation Officials
- 6. Micro Bridger (Version 1.0), U.S. Army Construction Engineering Research Laboratory

Issued 4/28/95 Rev. 0 16 BRIDGES - Page - 56

#### LEVEL III GUIDE SHEET - KEY NO. 3

**COMPONENT:** 

STEEL PILES

**CONTROL NUMBER:** 

GS-III 16.02.03-3

#### **Application**

This guide has been prepared to identify the purpose of a Level III inspection and its more sophisticated test and inspection methods which may be appropriate to determine the cause and/or extent of defects recorded in Level I or Level II defect observations at a steel pile.

Whereas the purpose of the Level I inspection was to record the observable defects at the piles, this Level III inspection is performed to provide a thorough systematic evaluation of the observed defect and to make an assessment of it's effects, if left unchecked, on the safety, durability and stability of the piles and it's appurtenant works.

The Level III inspection should be performed when prompted by the results of a Level I or II inspection. The inspection should be performed by an engineer or multidisciplined team of engineers experienced in the design and construction of bridges.

The results of the Level III inspection will be used to develop maintenance or remedial measure work strategy that will correct the existing deficiency conditions or to require continued monitoring of existing deficiency conditions at the piles.

In general, appropriate advanced inspection methods will be identified, recommended, and performed by or under the supervision of the inspection engineer personnel as part of the Level III test and inspection method. Advanced inspection methods will be assigned only after the assessment of defect conditions observed during a Level I or II inspection.

### **Special Safety Requirements**

Passing traffic is a hazard. Level III inspection and testing must be performed with the prior approval of the Facility Manager who will notify the authorities to provide safety measures and safe access.

#### LEVEL III GUIDE SHEET - KEY NO. 3 (Continued)

COMPONENT:

STEEL PILES

**CONTROL NUMBER:** 

GS-III 16.02.03-3

#### **Inspection Action**

Prior to making a field inspection of the observed defect, review all past records
concerning the piles and the defective component if available. These records may
include pre-construction investigation records, design criteria and analysis records,
available construction records, previous periodic maintenance inspection records,
water level records, and photographs taken during initial construction and
subsequent inspections.

- 2. Perform inspection of the pertinent components where observed defects that triggered a Level III inspection are listed.
- Make an assessment of the importance of individual defects observed for a given component at the bridge site. Indicate priorities for any required maintenance, or remedial measure work.
- 4. Identify whether particular observed defects need additional or continued observation.
- 5. Assess the stability and safety of the piles.
- 6. Prepare final cost estimate for advanced inspection methods required to determine the cause and extent of the observed defect.
- 7. Prepare cost estimate for required maintenance or remedial repair measures, as applicable.

Level III advanced inspection methods may be required for several Level I and Level II defect conditions observed at a bridge site. Level III advanced test or inspection methods and associated observed defects for steel piles include, but are not limited to the following:

# **LEVEL III GUIDE SHEET - KEY NO. 3 (Continued)**

**COMPONENT:** 

STEEL PILES

**CONTROL NUMBER:** 

GS-III 16.02.03-3

#### 

1. Grinding and or sandblasting, using caliper to measure section

corrosion of steel and section loss

loss

2. Magnetic particle

cracks in steel or welds

3. Dye-Penetrant

cracks in steel or welds

4. Ultrasonic test

cracks and voids in steel

5. Soil boring

Soil instability, movement and settlement

6. Laboratory test on soil sample (Strength tests, moisture content, consolidation test, etc)

Soil instability

7. Underwater Inspection

erosion, scouring and undermining

8. Survey measurements

pile movement

#### **Special Instructions**

Review as-built and design drawings of structure.

#### **Special Tools & Equipment Requirements**

Grinder or sandblasting equipment
Surveying equipment
Navigable boat and related safety equipment
Industry required testing equipment needed to perform the advanced investigation method chosen

#### Recommended Inspection Frequency

Triggered by Level I and Level II defect/observations or every 3 years.

### LEVEL III GUIDE SHEET - KEY NO. 3 (Continued)

COMPONENT:

STEEL PILES

**CONTROL NUMBER:** 

GS-III 16.02.03-3

# **References**

1. U.S. Department of transportation, Federal Highway Administration, Bureau of Public roads (DOT) Bridge Inspector's Training Manual, 1990 Edition

- 2. Bridge inspection and Rehabilitation, Parsons Brinckerhoff Edited by L.G. Silano, PE, 1993
- 3. Inspection of Bridges and Trestles NAVFAC MO-126, October 1991
- 4. AASHTO Manual for Maintenance Inspection of Bridges, American Association of State Highway and Transportation Officials
- 5. Micro Bridger (Version 1.0), U.S. Army Construction Engineering Research Laboratory

#### **16.03 PIERS**

# **DESCRIPTION**

A pier is an intermediate substructure unit located between the ends of a bridge. Its function is to support the superstructure at intermediate intervals with minimal obstruction to the flow of traffic or water. The most common pier types are: solid shaft, columns, columns with web wall, and cantilever. The primary material used in these constructions are plain cement concrete, reinforced concrete, stone masonry, timber, steel or a combination of these materials.

# SPECIAL TOOL AND EQUIPMENT REQUIREMENTS

The following list of special tools and equipment, beyond the requirements listed in the Standard Tool Section, may be required to perform the inspection of this subsystem.

- 1. Boat
- 2. Related safety equipment
- 3. Diving gear and related safety equipment

#### SPECIAL SAFETY REQUIREMENTS

Level I and Level II inspections are performed by walking the perimeter of the bridge pier and observing defects from ground level, utilizing binoculars where required thus passing traffic is a hazard. The inspection must be performed with prior approval of the Facility Manager, who will notify the necessary authorities so as to provide traffic safety measures and access. It is suggested that the inspector wear an orange safety vest.

Depending on the bridge type a boat may be required to observe certain components, and the inspectors are required to take all necessary safety measures and refer to the Master Safety Plan for guidance and compliance. The underwater inspection must be accomplished by a certified diver, as indicated in the introduction of this book, and in the Level III key description.

### **COMPONENT LIST**

- ◆ 16.03.01 TIMBER PIER
- ◆ 16.03.02 CONCRETE PIER
- ◆ 16.03.03 MASONRY PIER
- ◆ 16.03.04 STEEL PIER

#### **RELATED SUBSYSTEMS**

Due to the related nature of the elements requiring inspection, the following should be reviewed for concurrent inspection activities.

13.02	RETAINING WALLS
19.01	ROADWAYS
20.01	RAILROAD
21.00	WATERFRONT (all subsystems)

#### 16.03 PIERS

#### STANDARD INSPECTION PROCEDURE

The Level I inspection shall be carried out for each component listed. The inspector will identify any physical defects existing in each component; will note the level of severity of each defect type; and measure the quantity of each defect.

#### **COMPONENTS**

#### ◆ 16.03.01 TIMBER PIER

A timber pier consist of a footing, vertical elements (two or more rows of columns or posts), pier cap, and longitudinal and transverse bracing. The footing transmit the pier loads to the soil, rock or to some other foundation unit such as piles, caissons or drilled shaft. Since the foundation (footing and piles) are buried, this component cannot be inspected. The pile cap receives and distributes the superstructure loads to columns which are transmitted to the footing. Both above and underwater inspection portions of the piers shall be inspected. Underwater inspection by diver(s) shall be a Level III inspection.

The potential defects which may be observed include erosion or scouring, vertical, lateral and rotational movement due to soil instability and deterioration to timber members.

Defect:		UOM	LEVEL II KEY	LEVEL III KEY
* Bearing Seat - Pier Cap:				
Observation:				
<ul><li>a. Moist and staine</li><li>***{Severity L}</li></ul>	d, surface solid.	SF		
b. Dirt and debris a on bearing seat.	ccumulated	SF		
* * * {Severity L}				
c. Moist and staine soft, slight crush	=	SF	1	
* * * {Severity M}	· ·			
d. Area soft and cr seriously deterio		SF		1
***{Severity H}				
Defect:				
* Splits - Cap, Columns at	nd Bracing:			

Observation:		
a. Partial split in member.	LF	
* * * {Severity L}		
b. Split completely through member.	LF	
* * * {Severity M}		
c. Member split and failed.	LF	1
* * * {Severity H}		

16 BRIDGES - Page - 62 Issued 4/28/95 Rev. 0

# 16.03 PIERS

# **COMPONENTS (Continued)**

# ◆ 16.03.01 TIMBER PIER (Continued)

Defect:	UOM	LEVEL II KEY	LEVEL III KEY
* Decay - Pier Cap, Columns and Joints:  Decay from rot/fungus is most likely to occur at connections, splices, support points or around bolt holes. This may be due either to the tendency of such areas to collect and retain moisture, or to bolts, holes or cuts being made in the surface after the preservative treatment has been applied.  Observation:			
<ul> <li>a. Moist and stain or discolored area,</li> <li>signs of fungi, surface is</li> <li>solid.</li> <li>*** {Severity L}</li> </ul>	SF		
<ul><li>b. Surface spongy, member may shown signs of crushing</li><li>***{Severity M}</li></ul>	SF	1	
<ul> <li>c. Brown and white - discolored area, member may show section loss and crushing.</li> <li>***{Severity H}</li> </ul>	SF		1
Defect:			
* Weathering - Cap, Columns and Bracing: Observation:			
<ul> <li>a. Surface of wood is rough and corrugated and member may be warped.</li> <li>***{Severity L}</li> </ul>	SF		
b. Surface of wood is rough and corrugated with cracks partially through the wood member, may have minor section loss. Member may be warped.  ***{Severity M}	SF	1	
<ul><li>c. Large cracks extend deeply or completely through the wood.</li><li>***{Severity H}</li></ul>	SF		1
<ul><li>d. Wood is crumbly and seriously deteriorated.</li><li>***{Severity H}</li></ul>	SF		1

COMPONENTS (Continued)	7000		
◆ 16.03.01 TIMBER PIER (Continued)			
Defect:	иом	LEVEL II KEY	LEVEL III KEY
* Parasites - Pier Cap, Columns and Bracing: (termites, carpenter ants, powder post beetles) Observation:			
<ul><li>a. Pinholes with dark stained area around the holes.</li><li>***{Severity L}</li></ul>	SF		
<ul><li>b. Holes less than 1/8" dia., surface sag, and sawdust observed.</li><li>***{Severity M}</li></ul>	SF	1 .	
<ul> <li>c. Holes greater than 1/8" dia., surface channels, and crushing of the member.</li> <li>***{Severity H}</li> </ul>	SF		1
Defect:			
* Connections:			
Observation: a. Loose fasteners.	EA		
***{Severity L}	LA		
<ul><li>b. Member broken, spilt or damage.</li><li>* * * {Severity H}</li></ul>	LF		
<ul><li>c. Missing fastener or anchors.</li><li>***{Severity H}</li></ul>	LF		
Defect:			
* Collision Damage:			
Observation: a. Member out-of-alignment. * * * {Severity H}	LF		1
b. Member cracked, crushed or missing.  ***{Severity H}	LF		1
Defect:			
* Scouring or Erosion - Base of Footing: Observation:			
<ul><li>a. Voids less than 2".</li><li>***{Severity L}</li></ul>	SF		
b. Voids greater than 2", less than 6".  ***{Severity H}	SF		
<ul><li>c. Undermining of footing, with voids greater than 6".</li><li>***{Severity H}</li></ul>	SF		1

# **COMPONENTS (Continued)**

## ◆ 16.03.01 TIMBER PIER (Continued)

Defect:		UOM	LEVEL II KEY	LEVEL III KEY
* D	eflection Pier Cap:			
	Observation:			
	<ul> <li>Slight deflection of member when vehicle passes.</li> </ul>	LF		
	* * * {Severity L}			
	b. Noticeable deflection of member when vehicle passes.	LF	•	
	* * * {Severity M}			
	c. Large deflection of member when vehicle passes.	LF		1
	* * * {Severity H}			
	<ul><li>d. Permanent deformation in member.</li><li>***{Severity H}</li></ul>	LF		1
Defect:				
* R	otational Movement:			

_		
7	bservation	٠.

a. Pier rotated or tipping
\*\*\*{Severity H} EΑ 1

#### **COMPONENTS (Continued)**

#### **◆ 16.03.02 CONCRETE PIER**

A concrete pier consist of a footing, vertical elements (columns, solid shaft or columns with web wall) and pier cap. The footing transmit the pier loads to the soil, rock or to some other foundation unit such as piles, caissons or drilled shaft. Since the foundation (footing and piles) are buried, this component cannot be inspected. The pile cap receives and distributes the superstructure loads to the columns or shaft which are transmitted to the footing. Both above and underwater inspection portions of the piers shall be inspected. Underwater inspection by diver(s) shall be a Level III inspection.

Potential defects which may be observed include erosion or scour, vertical, lateral and rotational movement of the pier due to instability of the soil and deterioration of the concrete surfaces.

Defect:	иом	LEVEL II KEY	LEVEL III KEY
* Concrete Disintegration - Inspect Waterline, Splash Zone and Ground Line Areas:			
Observation:  a. Hollow spaces or voids present  within concrete aggregate exposed  concrete sound.  ***{Severity L}	SF		
<ul> <li>b. Hollow spaces or voids present         within concrete aggregate exposed         or missing.</li> <li>***{Severity M}</li> </ul>	SF		
<ul> <li>c. Hollow spaces or voids present         within concrete with exposed         reinforcing bars.         ***{Severity H}</li> </ul>	SF		2
Defect:			
* Erosion or Scouring - Base of Pier: Observation:			
<ul><li>a. Voids less than 2".</li><li>***{Severity L}</li></ul>	SF		
b. Voids greater than 2", less than 6".  ***{Severity H}	SF		
<ul><li>c. Undermining of base voids greater than 6".</li><li>* * * {Severity H}</li></ul>	SF		2

### **COMPONENTS (Continued)**

### ◆ 16.03.02 CONCRETE PIER (Continued)

Defect:	UOM	LEVEL II KEY	LEVEL III KEY
* Horizontal Cracks - Pier Cap, Wall or Columns:			
Observation:			
<ul><li>a. Hairline cracks less than 1/16" wide</li><li>***{Severity L}</li></ul>	LF		
<ul><li>b. Medium crack greater than 1/16", less than 1/8" wide. Staining</li></ul>	LF	2	
of concrete surface with signs of efflorescence deposit and spalling of cracks.			
* * * {Severity M}			
c. Wide cracks greater than 1/8" wide. Staining of concrete surface with efflorescence deposit, spalling of cracks, and reinforcing bars exposed.	LF		2
* * * {Severity H}			
Defect:			
* Honeycombing:			
Observation:			
<ul> <li>Hollow spaces or voids present within concrete, aggregate partially exposed,</li> </ul>	SF		

a. Hollow spaces or voids present within concrete, aggregate partially exposed, concrete is sound around damaged area.
\*\*\*{Severity L}
b. Hollow spaces or voids present with concrete with exposed aggregate, concrete is sound around defected area.
\*\*\*{Severity M}
c. Hollow spaces or voids present within SF

c. Hollow spaces or voids present within concrete with exposed reinforcing bars.

\*\*\*{Severity H}

2

### **COMPONENTS (Continued)**

### 16.03.02 CONCRETE PIER (Continued)

Defect:	MOU	LEVEL II	LEVEL III
* Diagonal Cracks - Pier Cap, Wall or Columns: Observation:			
<ul><li>a. Hairline cracks less than 1/16" wide.</li><li>***{Severity L}</li></ul>	LF		
<ul> <li>b. Medium crack greater than 1/16", less than 1/8" wide. Staining of concrete surface with signs of efflorescence deposit and spalling of cracks.</li> </ul>	LF	2	
***{Severity M} c. Wide cracks greater than 1/8" wide. Staining of concrete surface with efflorescence deposit, spalling of cracks and reinforcing bars exposed.  ***{Severity H}	LF		2

### **Defect:**

### \* Vertical Cracks - Pier Cap, Wall, or Columns:

with efflorescence deposit, spalling of cracks and reinforcing bars exposed.

Observation:

a.	Hairline cracks less than 1/16" wide.	LF		
* * *	'{Severity L}			
b.	Medium crack greater than 1/16",	LF	2	
	less than 1/8" wide. Staining			
	of concrete surface with signs of			
	efflorescence deposit and spalling			
	of cracks.			
* * *	{Severity M}			
c.	Wide cracks greater than 1/8"	LF		
	wide. Staining of concrete surface			

\*\*\*{Severity H}

2

### **COMPONENTS (Continued)**

### **◆** 16.03.02 CONCRETE PIER (Continued)

Defect:	UOM	LEVEL II KEY	LEVEL III KEY
* Spalling - Pier Cap, Wall or Columns: Observation:			
<ul> <li>a. Depression less than 1" deep and less than 6" in diameter.</li> </ul>	SF		
* * * (Severity L)			
b. Depression greater than 1" deep and greater than 6" in diameter.	SF	2 .	
* * * {Severity M}			
<ul> <li>Depression greater than 1" deep and greater than 6" in diameter with corroded re-bars.</li> </ul>	SF		2
* * * {Severity H}			
Defect			

### **Defect:**

### \* Scaling - Pier Cap, Wall or Columns:

Observation:

a.	Loss of surface mortar greater than	SF
	1/4" deep and less than 1/2"	
	deep with exposed aggregate.	

\*\*\*{Severity L}

b. Loss of surface mortar greater than SF 1/2" deep and less than 1" deep. Coarse aggregates are clearly exposed.

\* \* \* {Severity M}

c. Loss of coarse aggregate particles, as well as mortar surrounding coarse aggregates; depth of the loss greater than 1" deep, reinforcing bars exposed.

\*\*\*{Severity H}

2

SF

·		
UOM	LEVEL II	LEVEL III KEY
LF		2
SF SF SF		2
LF LF		2
	LF SF SF	LF  LF  LF

## **COMPONENTS (Continued)**

## ◆ 16.03.02 CONCRETE PIER (Continued)

Defect:	UOM	LEVEL II KEY	LEVEL III KEY
* Construction Joints:			
Observation:			
<ul> <li>a. Joint separation or movement less than 1/16".</li> </ul>	LF		
* * * {Severity L}			
b. Joint separation or movement greater than 1/16", less than 1/4".	LF	•	
* * * {Severity M}			
<ul> <li>c. Joint separation or movement greater than 1/4".</li> </ul>	LF		2
* * * {Severity H}	•		

### **Defect:**

# \* Popout:

Observation:

a.	Conical shape holes less than	SF
	1/2" diameter.	
* *	*{Severity L}	
b.	Conical shape hole greater than	SF
	1/2" less than 2-1/2" diameter.	
* *	*{Severity M}	
c.	Conical shape hole greater than	SF

LEVEL III

**LEVEL II** 

### **16.03 PIERS**

#### **COMPONENTS (Continued)**

#### **◆ 16.03.03 MASONRY PIER**

A masonry pier consist of a footing and pier shaft. The footing transmit the pier loads to the soil, rock or to some other foundation unit such as piles, caissons or drill shaft. Since the foundation (footing and piles) are buried, this component cannot be inspected. The pier shaft receives and distributes the superstructure loads which are transmitted to the footing. Both above and underwater inspection portion of the piers shall be inspected. Underwater inspection by diver(s) shall be a Level III inspection.

Potential defects which may be observed include erosion or scour, vertical, lateral and rotational movement of the pier due to instability of the soil and deterioration of the concrete surfaces.

Defect:	UOM	KEY	KEY
* Erosion or Scouring - Base of Pier: Observation:			
<ul><li>a. Voids less than 2".</li><li>***{Severity L}</li></ul>	SF		
<ul><li>b. Voids greater than 2",</li><li>less than 6".</li><li>***{Severity H}</li></ul>	SF		
<ul><li>c. Undermining of base voids greater than 6".</li><li>* * * {Severity H}</li></ul>	SF		3
Defect:			
* Bearing Seat:			
Observation:  a. Light spalling and chipping  of masonry.	SF		
* * * {Severity L} b. Dirt and debris accumulated on bearing seat. * * * {Severity L}	SF		
c. Spalling, cracking of masonry at edge of seat.  ***{Severity M}	SF		
d. Severe spalling and cracking with crushing of masonry.  ***{Severity H}	SF		3

### **COMPONENTS (Continued)**

### ◆ 16.03.03 MASONRY PIER (Continued)

Defect:	UOM	LEVEL II KEY	LEVEL III KEY
* Mortar Joint:			
Observation:			
<ul> <li>a. Mortar joint cracked with no voids, masonry stone sound.</li> </ul>	SF		
* * * {Severity L}			
<ul> <li>b. Mortar joint deteriorated with voids, vegetation growing from joint, masonry stone loose.</li> <li>***{Severity M}</li> </ul>	SF		
c. Mortar joint totally deteriorated, masonry stone missing.	SF		3
* * * {Severity H}			
Defect:			
* Masonry Stone Deterioration:			
Observation:			

SF

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- a. Masonry stone, minor spalling SF and hairline cracks.
- \*\*\*{Severity L}
- b. Masonry stone, spalling with cracks SF and chipping, stone loose.
- \*\*\*{Severity M}
- c. Masonry stone, spalling with large cracks and chipping, section loss greater than 15%.
- \*\*\*{Severity H}

3

## **COMPONENTS (Continued)**

## ◆ 16.03.03 MASONRY PIER (Continued)

Defect:	UOM	LEVEL II KEY	LEVEL III KEY
* Rotational Movement: Observation: a. Pier rotated or tipping.	LF		3
***{Severity H}			J
Defect:			
* Collision Damage:			
Observation:			
a. Member out-of-alignment.	LF		3
* * * {Severity H}			
<ul> <li>b. Member cracked, crushed or missing.</li> </ul>	LF		3
* * * {Severity H}			

#### **COMPONENTS** (Continued)

### ♦ 16.03.04 STEEL PIER

A steel pier consist of a footing, vertical elements (two or more rows of columns), pier cap, and longitudinal and transverse bracing. The footing transmit the pier loads to the soil, rock or to some other foundation unit such as piles, caissons or drilled shaft. Since the foundation (footing and piles) are buried, this component cannot be inspected. The pile cap receives and distributes the superstructure loads to columns which are transmitted to the footing. Both above and underwater inspection portions of the piers shall be inspected. Underwater inspection by diver(s) shall be a Level III inspection.

Potential defects which may be observed includes corrosion, cracks due to out-of-plane distortion, collision damage and overload damage.

Defect:	UOM	KEY	KEY
* Corrosion - Pier Cap, Columns and Bracings: Observation:	,		
<ul><li>a. Surface rust no pitting evident.</li><li>***{Severity L}</li></ul>	LF		
<ul><li>b. Corrosion evident pitting and blistering of base material.</li><li>* * * {Severity M}</li></ul>	LF		
<ul><li>c. Corrosion evident with loss to base section metal.</li><li>***{Severity H}</li></ul>	LF		4
Defect:			
* Cracks - Pier Cap, Columns and Bracing: Observation:			
<ul><li>a. Hairline or greater crack,</li><li>fillet of flanges.</li><li>***{Severity H}</li></ul>	LF		4
b. Hairline or greater crack, fillet of web.  * * * {Severity H}	LF		4

## **COMPONENTS (Continued)**

# ◆ 16.03.04 STEEL PIER (Continued)

Defect:	иом	LEVEL II KEY	LEVEL III KEY
* Cracks - Attachment Welds: Check all vertical and longitudinal stiffener welds for cracks. Observation: a. Hairline or greater crack, at toe of weld or adjacent metal.  ***{Severity H}	LF		4
Defect:			
* Straightness or Buckling of Member:			
Observation:  a. Sign of wrinkles in web  and or stiffener plate at support.  ***{Severity M}	LF		
b. Sign of wrinkles in flanges.	LF		
***{Severity M} c. Sign of buckling in web and or stiffener plate at support.	LF		4
<pre>***{Severity H} d. Sign of buckling in flanges. ***{Severity H}</pre>	LF		4
Defect:			
* Deflection Pier Cap: Observation:			
<ul> <li>a. Slight deflection of member when vehicle passes.</li> </ul>	LF		
<ul><li>***{Severity L}</li><li>b. Noticeable deflection of member when vehicle passes.</li></ul>	LF		
<pre>***{Severity M} c. Large deflection of member when     vehicle passes. ***{Severity H}</pre>	LF		4
d. Permanent deformation of member.  ***{Severity H}	LF		4

### **COMPONENTS (Continued)**

## ◆ 16.03.04 STEEL PIER (Continued)

Defect:	UOM	LEVEL II KEY	LEVEL III KEY
* Corrosion - Gussets or Connection Plates:			
Observation:	05		
<ul><li>a. Surface rust no pitting evident.</li><li>***{Severity L}</li></ul>	SF		
b. Corrosion evident pitting and	SF		
blistering of base material.			
***{Severity M}			
<ul> <li>c. Corrosion evident with loss to base section.</li> </ul>	SF		4
* * * {Severity H}			
Defects:			
* Corrosion - Bolts or Fasteners:			
Observation:			
<ul><li>a. Surface rust no pitting evident.</li><li>***{Severity L}</li></ul>	EA		
b. Corrosion evident pitting and	EA		
blistering of base material.			
* * * {Severity M}			_
<ul> <li>c. Corrosion evident with loss to base section.</li> </ul>	EA		4
* * * {Severity H}			
Defect:			
Defect.			
* Connectors or Fasteners:			
Observation:	EA		
<ul><li>a. Loose bolts or fasteners.</li><li>***{Severity L}</li></ul>	EA		
b. Missing fasteners or connectors.	EA		
* * * {Severity H}			
c. Crack in weld.	LF		4
* * * {severity H} d. Crack in connection plate.	LF		4
* * * {Severity H}	L.I		<b>-r</b>

## **COMPONENTS (Continued)**

### ◆ 16.03.04 STEEL PIER (CONTINUE)

Defect:	иом	LEVEL II KEY	LEVEL III KEY
* Rotational Movement: Observation: a. Pier rotated or tipping. ***{Severity H}	LF		4
Defect:			
* Vehicular Damage: Observation:			
<ul><li>a. Pier column out of alignment.</li><li>***{Severity H}</li></ul>	LF		4
<ul><li>b. Bracing member out of alignment.</li><li>***{Severity H}</li></ul>	LF		4

### **REFERENCES**

- 1. U.S. Department of transportation, Federal Highway Administration, Bureau of Public roads (DOT) Bridge Inspector's Training Manual, 1990 Edition.
- 2. Bridge inspection and Rehabilitation, Parsons Brinckerhoff Edited by L.G. Silano, PE, 1993
- 3. Inspection of Bridges and Trestles NAVFAC MO-126, October 1991
- 4. ACI Manual of Concrete Practice 1989, Part 1 Materials and General Properties of Concrete
- 5. AASHTO Manual for Maintenance Inspection of Bridges, American Association of State Highway and Transportation Officials
- 6. Micro Bridger (Version 1.0), U.S. Army Construction Engineering Research Laboratory

LEVEL II KEY	GUIDE SHEET CONTROL NUMBER	
		•
1	GS-II 16.03.01-1	
2	GS-II 16.03.02-2	
<b>LEVEL III KEY GI</b>	JIDE SHEET CONTROL NUMBER	
1	GS-III 16.03.01-1	
2	GS-III 16.03.02-2	
3	GS-III 16.03.03-3	
4	GS-III 16.03.04-4	

### LEVEL II GUIDE SHEET - KEY NO. 1

**COMPONENT:** 

TIMBER PIER

**CONTROL NUMBER:** 

GS-II 16.03.01-1

### **Application**

This applies to the investigation of possible interior and exterior deterioration of timber piers due to decay (rot), or parasites.

The results of the Level II inspection can be used to trigger a level III inspection or necessary repair.

### **Special Safety Requirements**

No Special Safety Requirements, beyond the requirements listed in the Safety Section, are to be observed in the performance of the Level II.

### **Inspection Actions**

- 1. Clean affected area.
- 2. Measure affected area.
- 3. Tap with hammer to determine extend of hollow or sound material.
- 4. Probe with ice pick.

### **Recommended Inspection Frequency**

As triggered by Level I or Level II defect/observation.

#### References

- 1. U.S. Department of transportation, Federal Highway Administration, Bureau of Public roads (DOT) Bridge Inspector's Training Manual, 1990 Edition
- 2. Bridge inspection and Rehabilitation, Parsons Brinckerhoff Edited by L.G. Silano, PE, 1993
- 3. Inspection of Bridges and Trestles NAVFAC MO-126, October 1991
- 4. AASHTO Manual for Maintenance Inspection of Bridges, American Association of State Highway and Transportation Officials
- 5. Micro Bridger (Version 1.0), U.S. Army Construction Engineering Research Laboratory

### **LEVEL II GUIDE SHEET - KEY NO. 2**

COMPONENT: CONTROL NUMBER: **CONCRETE PIERS** 

GS-II 16.03.02-2

#### **Application**

This applies to the investigation of concrete bridge piers deterioration due to spalling from delamination.

The results of the Level II inspection can be used to trigger a level III inspection or necessary repair.

### **Special Safety Requirements**

No special Safety Requirements, beyond the requirements listed in the Safety Section, are to be observed in the performance of the level II.

### **Inspection Actions**

- 1. Clean loose concrete from area to be inspected.
- 2. Measure the affected area.
- Tap the affected area with a hammer to determine extent of unsound or hollow concrete.

### **Recommended Inspection Frequency**

As Triggered by a Level I or Level II defect/observation.

#### References

- 1. U.S. Department of transportation, Federal Highway Administration, Bureau of Public roads (DOT) Bridge Inspector's Training Manual, 1990 Edition
- 2. Bridge inspection and Rehabilitation, Parsons Brinckerhoff Edited by L.G. Silano, PE, 1993
- Inspection of Bridges and Trestles NAVFAC MO-126, October 1991
- ACI Manual of Concrete Practice 1989, Part 1 Materials and General Properties of concrete
- 5. AASHTO Manual for Maintenance Inspection of Bridges, American Association of State Highway and Transportation Officials
- 6. Micro Bridger (Version 1.0), U.S. Army Construction Engineering Research Laboratory

### **LEVEL III GUIDE SHEET - KEY NO. 1**

**COMPONENT:** 

**TIMBER PIERS** 

**CONTROL NUMBER:** 

GS-III 16.03.01-1

### **Application**

This guide has been prepared to identify the purpose of a Level III inspection and its more sophisticated test and inspection methods which may be appropriate to determine the cause and/or extent of defects recorded in Level I or Level II defect observations at a timber pier.

Whereas the purpose of the Level I inspection was to record the observable defects at the piers, this Level III inspection is performed to provide a thorough systematic evaluation of the observed defect and to make an assessment of it's effects, if left unchecked, on the safety, durability and stability of the piers and it's appurtenant works.

The Level III inspection should be performed when prompted by the results of a Level I or II inspection. The inspection should be performed by an engineer or multidisciplined team of engineers experienced in the design and construction of bridge.

The results of the Level III inspection will be used to develop maintenance or remedial measure work strategy that will correct the existing deficiency conditions or to require continued monitoring of existing deficiency conditions at the piers.

In general, appropriate advanced inspection methods will be identified, recommended, and performed by or under the supervision of the inspection engineer personnel as part of the Level III test and inspection method. Advanced inspection methods will be assigned only after the assessment of defect conditions observed during a Level I or II inspection.

### **Special Safety Requirements**

Passing traffic is a hazard. Level III inspection and testing must be performed with the prior approval of the Facility Manager who will notify the authorities to provide safety measures and safe access.

### LEVEL III GUIDE SHEET - KEY NO. 1 (Continued)

COMPONENT:

TIMBER PIERS

**CONTROL NUMBER:** 

GS-III 16.03.01-1

#### **Inspection Action**

- Prior to making a field inspection of the observed defect, review all past records
  concerning the piers and the defective component if available. These records may
  include pre-construction investigation records, design criteria and analysis records,
  available construction records, previous periodic maintenance inspection records,
  reservoir level records, and photographs taken during initial construction and
  subsequent inspections.
- 2. Perform inspection of the pertinent components where observed defects that triggered a Level III inspection are listed.
- Make an assessment of the importance of individual defects observed for a given component at the bridge site. Indicate priorities for any required maintenance, or remedial measure work.
- 4. Identify whether particular observed defects need additional or continued observation.
- 5. Assess the stability and safety of the piers.
- Prepare final cost estimate for advanced inspection methods required to determine the cause and extent of the observed defect.
- 7. Prepare cost estimate for required maintenance or remedial repair measures, as applicable.

Level III advanced inspection methods may be required for several Level I and Level II defect conditions observed at a bridge site. Level III advanced test or inspection methods and associated observed defects for piers include, but are not limited to the following:

### LEVEL III GUIDE SHEET - KEY NO. 1 (Continued)

**COMPONENT:** 

TIMBER PIERS

**CONTROL NUMBER:** 

GS-III 16.03.01-1

### 

1. Increment borer interior and exterior deterioration of timber due to

decay or parasites

2. Ultrasonic test interior deterioration

3. Moisture content deterioration due to decay or parasites

4. Soil borings soil instability, movement and settlement

Laboratory tests on soil samples (strength tests, moisture content,

consolidation tests, etc.)

soil instability

6. Underwater inspection

erosion, scouring and undermining

7. Survey measurement

pier movement

### **Special Instructions**

Review as-built and design drawings of structure.

### **Special Tools & Equipment Requirements**

Surveying equipment
Navigable boat and related safety equipment
Industry required testing equipment needed to perform the advanced investigation
method chosen

### **Recommended Inspection Frequency**

As triggered by Level I or Level II defect/observation or every 3 years.

### **LEVEL III GUIDE SHEET - KEY NO. 1 (Continued)**

COMPONENT:

**TIMBER PIERS** 

**CONTROL NUMBER:** 

GS-III 16.03.01-1

### **References**

1. U.S. Department of transportation, Federal Highway Administration, Bureau of Public roads (DOT) Bridge Inspector's Training Manual, 1990 Edition

- 2. Bridge inspection and Rehabilitation, Parsons Brinckerhoff Edited by L.G. Silano, PE, 1993
- 3. Inspection of Bridges and Trestles NAVFAC MO-126, October 1991
- 4. AASHTO Manual for Maintenance Inspection of Bridges, American Association of State Highway and Transportation Officials
- 5. Micro Bridger (Version 1.0), U.S. Army Construction Engineering Research Laboratory

#### LEVEL III GUIDE SHEET - KEY NO. 2

COMPONENT:

**CONCRETE PIERS** 

**CONTROL NUMBER:** 

GS-III 16.03.02-2

#### **Application**

This guide has been prepared to identify the purpose of a Level III inspection and its more sophisticated test and inspection methods which may be appropriate to determine the cause and/or extent of defects recorded in Level I or Level II defect observations at a concrete pier.

Whereas the purpose of the Level I inspection was to record the observable defects at the piers, this Level III inspection is performed to provide a thorough systematic evaluation of the observed defect and to make an assessment of it's effects, if left unchecked, on the safety, durability and stability of the piers and it's appurtenant works.

The Level III inspection should be performed when prompted by the results of a Level I or II inspection. The inspection should be performed by an engineer or multidisciplined team of engineers experienced in the design and construction of bridges.

The results of the Level III inspection will be used to develop maintenance or remedial measure work strategy that will correct the existing deficiency conditions or to require continued monitoring of existing deficiency conditions at the piers.

In general, appropriate advanced inspection methods will be identified, recommended, and performed by or under the supervision of the inspection engineer personnel as part of the Level III test and inspection method. Advanced inspection methods will be assigned only after the assessment of defect conditions observed during a Level I or II inspection.

### **Special Safety Requirements**

Passing traffic is a hazard. Level III inspection and testing must be performed with the prior approval of the Facility Manager who will notify the authorities to provide safety measures and safe access.

### LEVEL III GUIDE SHEET - KEY NO. 2 (Continued)

COMPONENT:

**CONCRETE PIERS** 

**CONTROL NUMBER:** 

GS-III 16.03.02-2

#### Inspection Action

Prior to making a field inspection of the observed defect, review all past records
concerning the piers and the defective component if available. These records may
include pre-construction investigation records, design criteria and analysis records,
available construction records, previous periodic maintenance inspection records,
water level records, and photographs taken during initial construction and
subsequent inspections.

- 2. Perform inspection of the pertinent components where observed defects that triggered a Level III inspection are listed.
- Make an assessment of the importance of individual defects observed for a given component at the bridge site. Indicate priorities for any required maintenance, or remedial measure work.
- 4. Identify whether particular observed defects need additional or continued observation.
- 5. Assess the stability and safety of the piers.
- Prepare final cost estimate for advanced inspection methods required to determine the cause and extent of the observed defect.
- 7. Prepare cost estimate for required maintenance or remedial repair measures, as applicable.

Level III advanced inspection methods may be required for several Level I and Level II defect conditions observed at a bridge site. Level III advanced test or inspection methods and associated observed defects for piers include, but are not limited to the following:

### LEVEL III GUIDE SHEET - KEY NO. 2 (Continued)

**COMPONENT:** 

**CONCRETE PIERS** 

**CONTROL NUMBER:** 

GS-III 16.03.02-2

### Advanced Test or Inspection Method

### **Applicable Observed Defects**

1. Infrared thermography and ground probing radar

concrete cracking

2. Concrete coring

concrete deterioration

 Laboratory tests on concrete (core, strength tests, abrasion, absorption,sulfate soundness, unit weight concrete deterioration

4. Ultrasonic test

cracks and voids in concrete

5. Half-cell test

corrosion to reinforcement steel

6. Soil borings

soil instability, movement and settlement

7. Laboratory tests on soil samples (strength tests, moisture content, consolidation tests, etc.)

soil instability

8. Underwater inspection

erosion, scouring and undermining

9. Survey measurements

pier movement

### **Special Instructions**

Review as-built and design drawings of structure.

#### **Special Tools & Equipment Requirements**

Surveying equipment
Navigable boat with related safety equipment
Industry required testing equipment needed to perform the advanced investigation
method chosen

### **Recommended Inspection Frequency**

As triggered by Level II or Level II defect/observation or every 3 years

### LEVEL III GUIDE SHEET - KEY NO. 2 (Continued)

COMPONENT:

**CONCRETE PIERS** 

**CONTROL NUMBER:** 

GS-III 16.03.02-2

#### References

1. U.S. Department of transportation, Federal Highway Administration, Bureau of Public roads (DOT) Bridge Inspector's Training Manual, 1990 Edition

- 2. Bridge inspection and Rehabilitation, Parsons Brinckerhoff Edited by L.G. Silano, PE, 1993
- 3. Inspection of Bridges and Trestles NAVFAC MO-126, October 1991
- 4. ACI Manual of Concrete Practice 1989, Part 1 Materials and General Properties of Concrete
- 5. AASHTO Manual for Maintenance Inspection of Bridges, American Association of State Highway and Transportation Officials
- 6. Micro Bridger (Version 1.0), U.S. Army Construction Engineering Research Laboratory

### **LEVEL III GUIDE SHEET - KEY NO. 3**

COMPONENT:

**MASONRY PIERS** 

**CONTROL NUMBER:** 

GS-III 16.03.03-3

#### **Application**

This guide has been prepared to identify the purpose of a Level III inspection and its more sophisticated test and inspection methods which may be appropriate to determine the cause and/or extent of defects recorded in Level I or Level II defect observations at a masonry pier.

Whereas the purpose of the Level I inspection was to record the observable defects at the piers, this Level III inspection is performed to provide a thorough systematic evaluation of the observed defect and to make an assessment of it's effects, if left unchecked, on the safety, durability and stability of the piers and it's appurtenant works.

The Level III inspection should be performed when prompted by the results of a Level I or II inspection. The inspection should be performed by an engineer or multidisciplined team of engineers experienced in the design and construction of bridges.

The results of the Level III inspection will be used to develop maintenance or remedial measure work strategy that will correct the existing deficiency conditions or to require continued monitoring of existing deficiency conditions at the piers.

In general, appropriate advanced inspection methods will be identified, recommended, and performed by or under the supervision of the inspection engineer personnel as part of the Level III test and inspection method. Advanced inspection methods will be assigned only after the assessment of defect conditions observed during a Level I or II inspection.

### **Special Safety Equipment**

Special safety equipment needed for the Level III inspection of masonry piers are listed in the standards developed for the Standard Inspection of piers.

#### **Special Safety Requirements**

Special safety requirements are as set forth in the standards developed for the Standard Inspection of masonry piers.

### **LEVEL III GUIDE SHEET - KEY NO. 3 (Continued)**

COMPONENT:

**MASONRY PIERS** 

**CONTROL NUMBER:** 

GS-III 16.03.03-3

### **Inspection Action**

- Prior to making a field inspection of the observed defect, review all past records
  concerning the piers and the defective component if available. These records may
  include pre-construction investigation records, design criteria and analysis records,
  available construction records, previous periodic maintenance inspection records,
  water level records, and photographs taken during initial construction and
  subsequent inspections.
- 2. Perform inspection of the pertinent components where observed defects that triggered a Level III inspection are listed.
- 3. Make an assessment of the importance of individual defects observed for a given component at the bridge site. Indicate priorities for any required maintenance, or remedial measure work.
- 4. Identify whether particular observed defects need additional or continued observation.
- 5. Assess the stability and safety of the piers.
- 6. Prepare final cost estimate for advanced inspection methods required to determine the cause and extent of the observed defect.
- 7. Prepare cost estimate for required maintenance or remedial repair measures, as applicable.

Level III advanced inspection methods may be required for several Level I and Level II defect conditions observed at a bridge site. Level III advanced test or inspection methods and associated observed defects for piers include, but are not limited to the following:

### **LEVEL III GUIDE SHEET - KEY NO. 3 (Continued)**

COMPONENT:

**MASONRY PIERS** 

**CONTROL NUMBER:** 

GS-III 16.03.03-3

### Advanced Test or Inspection Method

### Applicable Observed Defects

1. Infrared Thermography and ground probing radar

voids in masonry abutments

2. Ultrasonic test

cracks and voids in masonry

3. Soil borings

soil instability, movement and settlement

4. Laboratory tests on soil samples (strength tests, moisture content, consolidation tests, etc.)

soil instability

5. Underwater inspection

erosion, scouring and undermining

6. Survey measurement

pier movement

### **Special Instructions**

Review as-built and design drawings of structure.

### **Special Tools & Equipment Requirements**

Survey Level and rod Navigable boat with related safety equipment Industry required testing equipment needed to perform the advanced investigation method chosen

### **Recommended Inspection Frequency**

As triggered by Level I or Level II defect/observation or every 3 years

### LEVEL III GUIDE SHEET - KEY NO. 3 (Continued)

COMPONENT: CONTROL NUMBER: **MASONRY PIERS** 

GS-III 16.03.03-3

### References

1. U.S. Department of transportation, Federal Highway Administration, Bureau of Public roads (DOT) Bridge Inspector's Training Manual, 1990 Edition

- 2. Bridge inspection and Rehabilitation, Parsons Brinckerhoff Edited by L.G. Silano, PE, 1993
- 3. Inspection of Bridges and Trestles NAVFAC MO-126, October 1991
- 4. ACI Manual of Concrete Practice 1989, Part 1 Materials and General Properties of Concrete
- 5. AASHTO Manual for Maintenance Inspection of Bridges, American Association of State Highway and Transportation Officials
- 6. Micro Bridger (Version 1.0), U.S. Army Construction Engineering Research Laboratory

Issued 4/28/95 Rev. 0 16 BRIDGES - Page - 94

### LEVEL III GUIDE SHEET - KEY NO. 4

COMPONENT:

STEEL PIERS

**CONTROL NUMBER:** 

GS-III 16.03.04-4

#### **Application**

This guide has been prepared to identify the purpose of a Level III inspection and its more sophisticated test and inspection methods which may be appropriate to determine the cause and/or extent of defects recorded in Level I or Level II defect observations at a steel pier.

Whereas the purpose of the Level I inspection was to record the observable defects at the piers, this Level III inspection is performed to provide a thorough systematic evaluation of the observed defect and to make an assessment of it's effects, if left unchecked, on the safety, durability and stability of the piers and it's appurtenant works.

The Level III inspection should be performed when prompted by the results of a Level I or II inspection. The inspection should be performed by an engineer or multidisciplined team of engineers experienced in the design and construction of bridges.

The results of the Level III inspection will be used to develop maintenance or remedial measure work strategy that will correct the existing deficiency conditions or to require continued monitoring of existing deficiency conditions on the bridge piers.

In general, appropriate advanced inspection methods will be identified, recommended, and performed by or under the supervision of the inspection engineer personnel as part of the Level III test and inspection method. Advanced inspection methods will be assigned only after the assessment of defect conditions observed during a Level I or II inspection.

### **Special Safety Requirements**

Passing traffic is a hazard. Level III inspection and testing must be performed with the prior approval of the Facility Manager who will notify the authorities to provide safety measures and safe access.

### LEVEL III GUIDE SHEET - KEY NO. 4 (Continued)

COMPONENT:

STEEL PIERS

**CONTROL NUMBER:** 

GS-III 16.03.04-4

#### **Inspection Action**

- Prior to making a field inspection of the observed defect, review all past records
  concerning the piers and the defective component if available. These records may
  include pre-construction investigation records, design criteria and analysis records,
  available construction records, previous periodic maintenance inspection records,
  water level records, and photographs taken during initial construction and
  subsequent inspections.
- 2. Perform inspection of the pertinent components where observed defects that triggered a Level III inspection are listed.
- Make an assessment of the importance of individual defects observed for a given component at the bridge site. Indicate priorities for any required maintenance, or remedial measure work.
- 4. Identify whether particular observed defects need additional or continued observation.
- 5. Assess the stability and safety of the piers.
- 6. Prepare final cost estimate for advanced inspection methods required to determine the cause and extent of the observed defect.
- 7. Prepare cost estimate for required maintenance or remedial repair measures, as applicable.

Level III advanced inspection methods may be required for several Level I and Level II defect conditions observed at a bridge site. Level III advanced test or inspection methods and associated observed defects for bridge piers include, but are not limited to the following:

Issued 4/28/95 Rev. 0

### LEVEL III GUIDE SHEET - KEY NO. 4 (Continued)

**COMPONENT:** 

STEEL PIERS

**CONTROL NUMBER:** 

GS-III 16.03.04-4

#### **Applicable Observed Defects** Advanced Test or Inspection Method

1. Grinding or sandblasting, using caliper to measure

corrosion of steel and section loss

section loss

2. Magnetic particle cracks in steel or welds

3. **Dye-Penetrant**  cracks in steel or welds

Ultrasonic test

cracks and voids in steel

5. Soil boring soil instability, movement and settlement

6. Laboratory test on soil sample (Strength tests, moisture content, consolidation test, etc)

soil instability

7. **Underwater Inspection** 

erosion, scouring undermining

8. Survey measurement pier movement

#### Special Instructions

Review as-built and design drawings of structure.

#### **Special Tools & Equipment Requirements**

Grinder or sandblasting equipment

Navigable boat with related safety equipment

Surveying equipment

Industry required testing equipment needed to perform the advanced investigation

method chosen

### Recommended Inspection Frequency

As triggered by Level I or Level II defect/observation or every 3 years

### **LEVEL III GUIDE SHEET - KEY NO. 4 (Continued)**

COMPONENT:

STEEL PIERS

**CONTROL NUMBER:** 

GS-III 16.03.04-4

### References

1. U.S. Department of transportation, Federal Highway Administration, Bureau of Public roads (DOT) Bridge Inspector's Training Manual, 1990 Edition

- 2. Bridge inspection and Rehabilitation, Parsons Brinckerhoff Edited by L.G. Silano, PE, 1993
- Inspection of Bridges and Trestles NAVFAC MO-126, October 1991
- 4. ACI Manual of Concrete Practice 1989, Part 1 Materials and General Properties of Concrete
- 5. AASHTO Manual for Maintenance Inspection of Bridges, American Association of State Highway and Transportation Officials
- 6. Micro Bridger (Version 1.0), U.S. Army Construction Engineering Research Laboratory

### **16.04 SUPERSTRUCTURE**

#### **DESCRIPTION**

The superstructure is designed to carry dead and live loads associated with the structural deck to the substructure (abutments or piers). The superstructure includes primary members; floor system with two or more main supporting members, secondary members, connections and bearings.

The floor system supports the deck or driving surface. It may consist of either closely spaced transverse floor beams or several longitudinal stringers carried by transverse floor beams. In floor systems of this type, stringers are usually wide flange beams, and the floor beams may be either plate girders, wide flange beams, or trusses. When floor beams only are used, they may be rolled or plate girders.

The main supporting members may be steel, timber or concrete beams; steel plate girders, steel or timber trusses; steel or concrete rigid frames. Beams and girders are considered single elements while trusses have several identifiable parts: the chords, which are generally longitudinal members at the top and bottom of a truss, and the verticals and diagonals which are called web members.

Secondary members for beams and girder structures are bracing which include diaphragms and cross frames. Trusses are braced with portal cross frames, and sway bracing. Diaphragms and cross frames stabilize the beams or trusses and distributes loads between them. A diaphragm is usually a solid web member, either a rolled shape or built-up member, while a cross frame is a truss panel, or frame.

The beams, girders, stringers, trusses and other members which form a complete bridge superstructure are designed to support certain loads. Each of these members must transmit its load through connections to supporting members. As a means to transmit this load fasteners such as bolts or welds are used with connection material, made of angles, plates or pieces of rolled sections.

Bearings transmit and distribute the superstructure loads to the substructure, and they permit the superstructure to undergo necessary movement without developing harmful overstresses.

#### SPECIAL TOOL AND EQUIPMENT REQUIREMENTS

The following list of special tools and equipment, beyond the requirements listed in the Standard Tool Section, are required to perform the inspection of this subsystem.

- 1. Boat
- 2. Related safety equipment

### **SPECIAL SAFETY REQUIREMENTS**

Level I and Level II inspections are performed by walking under the superstructure and observing from ground level, utilizing a binocular where required thus passing traffic is a hazard. The inspection must be performed with prior approval of the Facility Manager, who will notify the necessary authorities so as to provide traffic safety measures and access. It is suggested that the inspector wear an orange safety vest.

Depending on the bridge type a boat may be required to observe certain components. The inspectors are required to take all necessary safety measures and refer to the Master Safety Plan for guidance and compliance.

#### **COMPONENT LIST**

- ◆ 16.04.01 TIMBER PRIMARY AND SECONDARY MEMBERS
- ◆ 16.04.02 CONCRETE PRIMARY AND SECONDARY MEMBERS
- ◆ 16.04.03 STEEL PRIMARY AND SECONDARY MEMBERS
- **◆** 16.04.04 CONNECTIONS
- ◆ 16.04.05 BEARINGS

# **RELATED SUBSYSTEMS**

Due to the related nature of the elements requiring inspection, the following should be reviewed for concurrent inspection activities.

19.01	ROADWAYS
20.01	RAILROAD
29.00	SITE ELECTRICAL (all subsystems)
13.02	RETAINING WALLS

# STANDARD INSPECTION PROCEDURE

The Level I inspection shall be carried out for each component listed. The inspector will identify any physical defects existing in each component; will note the level of severity of each defect type; and measure the quantity of each defect.

### **COMPONENTS**

#### **◆ 16.04.01 TIMBER PRIMARY AND SECONDARY MEMBERS**

There are two basic classifications in timber bridge construction; solid sawn and glued-laminated. A solid sawn beam is simply a tree, with its bark and branches removed, and sawn down to the desired size. A glulam member is made by gluing strips of wood together to form a structural member of the desired size.

Potential defects which may be observed include; decay, parasites, vehicular damage, overload.

Defect:	UOM	LEVEL II- KEY	LEVEL III KEY
* Beam End - At Supports:			
Observation:			
<ul><li>a. Moist and stained, surface solid.</li><li>***{Severity L}</li></ul>	SF		
<ul> <li>b. Moist and stained, surface soft, beam slight crushing.</li> </ul>	SF	1	
* * * {Severity M}			
c. Area soft and crumbly and seriously deteriorated.	SF		1
* * * {Severity H}			

# Defect:

# \* Decay:

Decay from rot/fungus is most likely to occur at connections, splices, support points or around bolt holes. This may be due either to the tendency of such areas to collect and retain moisture, or to bolt holes or cuts being made in the surface after the preservative treatment has been applied.

#### Observation:

a.	Moist and stain or discolored area, signs of fungi, surface is	SF	
	solid.		
* * *	{Severity L}		
b.	Surface spongy, member may shown signs of crushing.	SF	1
* * *	{Severity M}		
C.	Brown and white - discolored area, member may show section loss and crushing.	SF	1
* * *	{Severity H}		

# **COMPONENTS (Continued)**

# ◆ 16.04.01 TIMBER PRIMARY AND SECONDARY MEMBERS (Continued)

Defect:	UOM	LEVEL II KEY	LEVEL III KEY
* Parasites:			
(Termites, carpenter ants, powder post beetles)			
Observation:			
<ul> <li>a. Pinholes with dark stain</li> <li>area around the holes.</li> </ul>	SF		
* * * {Severity L}			
<ul> <li>b. Holes less than 1/8" diameter, surface sag, and sawdust observed.</li> </ul>	SF	1	
* * * {Severity M}			
<ul> <li>c. Holes greater than 1/8" diameter, surface channels, and crushing of the member.</li> <li>***{Severity H}</li> </ul>	SF		1
Defect:			
* Horizontal Splits:			
Observation:			

# D

Observation:

a. Partial splits in member.	LF	
* * * {Severity M}		
b. Split completely through member.	LF	
* * * {Severity H}		
<ul> <li>Member split and completely failed.</li> </ul>	LF	1
* * * {Severity H}		

# **COMPONENTS (Continued)**

# ◆ 16.04.01 TIMBER PRIMARY AND SECONDARY MEMBERS (Continued)

Defect:	UOM	LEVEL II KEY	LEVEL III KEY
* Vehicular Damage - Primary and Secondary Members:	· 		
Observation:			
<ul> <li>a. Member out of alignment.</li> </ul>	LF		1
* * * {Severity H}			
<ul> <li>b. Member split or broken</li> </ul>	LF		1
at cracks.			
* * * {Severity H}			
Defect:			
* Deflection:			
Observe deflection of members with passing traffic.			
Observation:			
<ul> <li>a. Slight deflection in member.</li> </ul>	LF		
* * * {Severity L}			
b. Noticeable deflection in member.	LF		
* * * {Severity M}			_
c. Permanent sagging	LF		1
or deflection in member.			
***{Severity H}			
Defect:			
* Straightness:			
Observation:			
a. Slight bowing in member.	LF		
* * * {Severity L}			
b. Noticeable bowing in member.	LF		
* * * {Severity M}			
c. Excessive bowing in member.	LF		1
* * * {Severity H}			

#### **COMPONENTS (Continued)**

#### **◆ 16.04.02 CONCRETE PRIMARY AND SECONDARY MEMBERS**

Concrete beams and primary members transfer dead and live loads to the substructure (abutments or piers). The most common concrete bridges are the slab, T-beam, I-beam and box girder types. Concrete superstructure are classified according to the method of construction, cast-in-placed or precast and the method of reinforcement, conventional (mild steel).

Potential defects which may be observed include; cracking, scaling, delamination, spalling, honeycombs, collision damage, overload damage and reinforcing steel corrosion.

Defect:	UOM	LEVEL II KEY	LEVEL III KEY
* Beam End - At supports:			
Observation:			
<ul><li>a. Light spalling and chipping of concrete.</li><li>***{Severity L}</li></ul>	LF		
<ul><li>b. Spalling and cracks of beam.</li><li>***{Severity M}</li></ul>	LF		2
<ul> <li>c. Spalling, cracks and crushing, at end of beam.</li> </ul>	LF		2
* * * {Severity H}			
Defect: The second of the seco			
* Horizontal Cracks:			
Observation:			
<ul> <li>Hairline cracks less than 1/16 wide, slight staining of concrete surface.</li> </ul>	LF		
* * * {Severity L}			
b. Medium crack greater than 1/16" less, than 1/8" wide. Staining of concrete surface with signs of efflorescence deposit and spalling of cracks.	LF	2	
***{Severity M}			
c. Wide cracks greater than 1/8" wide. Staining of concrete surface with signs of efflorescence deposit, spalling of cracks and reinforcing bars exposed.  ***{Severity H}	LF		2

LEVEL III

LEVEL II

# **16.04 SUPERSTRUCTURE**

# **COMPONENTS (Continued)**

# **◆ 16.04.02 CONCRETE PRIMARY AND SECONDARY MEMBERS (Continued)**

Defect:	UOM	KEY	KEY
* Diagonal Cracks - Near supports: Observation:			
<ul> <li>Hairline cracks less than 1/16</li> <li>wide, slight staining of concrete surface.</li> </ul>	LF		
***{Severity L} b. Medium cracks greater than 1/16" less than " 1/8" wide. Staining of concrete surface with signs of efflorescence deposit and spalling of cracks.	LF	2	
***{Severity M} c. Wide cracks greater than 1/8" wide. Staining of concrete surface with signs of efflorescence deposit, spalling of cracks, and reinforcing bars exposed.  ***{Severity H}	LF		2
Defect:			
Vertical Cracks: Observation:			
<ul> <li>Hairline cracks less than 1/16"</li> <li>wide, slight staining of concrete surface.</li> </ul>	LF		
***{Severity L}  b. Medium cracks greater than 1/16" less than 1/8" wide. Staining of concrete surface with signs of efflorescence deposit and spalling of cracks.	LF	2	
***{Severity M} c. Wide cracks greater than 1/8" wide. Staining of concrete surface with signs of efflorescence deposit, spalling of cracks, and reinforcing bars exposed.  ***{Severity H}	LF		2

# **COMPONENTS (Continued)**

# ◆ 16.04.02 CONCRETE PRIMARY AND SECONDARY MEMBERS (Continued)

Defect:		UOM	LEVEL II KEY	LEVEL III KEY
* S	caling:			
	Observation:			
	<ul> <li>a. Loss of surface mortar greater than</li> <li>1/4" deep and less than 1/2"</li> <li>deep with exposed aggregate.</li> </ul>	SF		
	* * * {Severity L}			
	<ul> <li>b. Loss of surface mortar greater than</li> <li>1/2" deep and less than 1" deep. Coarse aggregates are clearly exposed.</li> </ul>	SF		
	* * * {Severity M}			
	<ul> <li>c. Loss of coarse aggregate particles,         as well as mortar surrounding         coarse aggregates; depth of the loss         greater than 1" deep, reinforcing bars         exposed.</li> <li>***{Severity H}</li> </ul>	SF		
Defect:				
* H	oneycombing:			
•••	Observation:			
	<ul> <li>Hollow spaces or voids present within concrete, aggregated partially exposed, concrete is sound around damage area.</li> </ul>	SF	·	
	* * * {Severity L} b. Hollow spaces or voids present with	SF	2	
	<ul> <li>b. Hollow spaces or voids present with concrete with exposed aggregate, concrete is sound around defected area.</li> <li>***{Severity M}</li> </ul>	<b>3</b> F	2	
	c. Hollow spaces or voids present within concrete with exposed rebars.	SF		2
Defect:				
* Vehicu	lar Damage - Primary and Secondary Members: Observation:			
	a. Member out of alignment.	LF		2
	* * * {Severity H}			
	<ul><li>b. Member cracked with section loss</li><li>***{Severity H}</li></ul>	LF		2

# **COMPONENTS (Continued)**

# **◆ 16.04.02 CONCRETE PRIMARY AND SECONDARY MEMBERS (Continued)**

Defect:	иом	LEVEL II KEY	LEVEL III KEY
* Spalling:			
Observation:			
a. Depression less than 1" deep and less than 6" in diameter.	SF		
***{Severity L} b. Depression greater than 1" deep and greater than 6" in diameter.	SF	2	
* * * {Severity M}			
<ul> <li>c. Depression greater than 1" deep and greater than 6" in diameter with corroded reinforcing bars.</li> <li>***{Severity H}</li> </ul>	SF		2
Defect:			
* Deflection:			
Observe deflection of members with passing traffic. Observation:			
<ul><li>a. Slight deflection in member.</li><li>***{Severity L}</li></ul>	LF		
<ul><li>b. Noticeable deflection in member.</li><li>***{Severity M}</li></ul>	LF		
c. Permanent sagging	LF		2
or deflection in member.			
* * * {Severity H}			
Defect:			
* Popout:			
Observation:			
<ul><li>a. Conical shape holes less than</li><li>1/2" diameter.</li></ul>	SF		
* * * {Severity L}			
<ul><li>b. Conical shape hole greater than 1/2" less than 2-1/2" diameter.</li><li>***{Severity M}</li></ul>	SF		
c. Conical shape hole greater than	SF		
2-1/2" in diameter.  ***{Severity M}	<b>J.</b>		

LEVEL III

LEVEL II

# **16.04 SUPERSTRUCTURE**

# **COMPONENTS (Continued)**

#### **◆ 16.04.03 STEEL PRIMARY AND SECONDARY MEMBERS**

Steel is one of the most common materials used in superstructures. There are many different types of superstructures, including: rolled multi-beam, fabricated multi-girder, two girder, box girder, trusses, rigid frames and many more.

Potential defects which may be observed includes corrosion, fatigue cracking due to out-ofplane distortion, collision damage and overload damage.

Defect:	UOM	KEY	KEY
* Corrosion - Top and Bottom Flange: Observation:			
<ul><li>a. Surface rust no pitting evident.</li><li>***{Severity L}</li></ul>	LF		
<ul><li>b. Corrosion evident pitting and blistering of base material.</li><li>***{Severity M}</li></ul>	LF		
<ul><li>c. Corrosion evident with loss to base section.</li><li>***{Severity H}</li></ul>	LF		3
Defect:			
* Corrosion - Web Plate: Observation:			
<ul><li>a. Surface rust no pitting evident.</li><li>***{Severity L}</li></ul>	SF		
<ul><li>b. Corrosion evident pitting and blistering of base material.</li><li>***{Severity M}</li></ul>	SF		
<ul><li>c. Corrosion evident with loss to base section.</li><li>***{Severity H}</li></ul>	SF		3

# **COMPONENTS (Continued)**

# ◆ 16.04.03 STEEL PRIMARY AND SECONDARY MEMBERS (Continued)

Defect:	UOM	LEVEL II KEY	LEVEL III KEY
* Cracks:			
Observation:			
<ul> <li>a. Hairline or greater crack,</li> <li>fillet of top flange.</li> </ul>	LF		3
* * * {Severity H}			
<ul> <li>b. Hairline or greater crack,</li> <li>fillet of bottom flange.</li> </ul>	LF		3
* * * {Severity H}			
<ul> <li>c. Hairline or greater vertical crack, in web of beam.</li> </ul>	LF		3
* * * {Severity H}			
Defect:			
* Cracks - Attachment Welds: Check all vertical and longitudinal stiffener welds for cracks.			
Observation:			
a. Hairline or greater crack,	LF		3
at toe of weld or adjacent metal. ***{Severity H}			
Defect:			
* Straightness or Buckling:			
Observation:	LF		
<ul><li>a. Sign of wrinkles in web</li><li>and or stiffener plate at support.</li><li>***{Severity M}</li></ul>	LF		
b. Sign of wrinkles in flange.  ***{Severity M}	LF		
<ul> <li>c. Sign of buckling in web and or stiffener plate at support.</li> </ul>	LF		3
* * * (Severity H)			_
<ul><li>d. Sign of buckling in flange.</li><li>***{Severity H}</li></ul>	LF		3

# **COMPONENTS (Continued)**

# ◆ 16.04.03 STEEL PRIMARY AND SECONDARY MEMBERS (Continued)

Defect:		UOM	LEVEL II KEY	LEVEL III KEY
* Supp	oort Ends:			
Ol	oservation:			
a.	Slight wrinkles in web or flanges.	LF		
* :	**{Severity M}			
<b>b.</b>	Sign of buckling in web or flanges.	LF		3
* + 1	**{Severity H}			
Defect:				
* Vehi	cular Damage:			
Ol	oservation:			
a. *:	Primary member out of alignment.  **{Severity H}	LF		3
b.	Secondary member out-of alignment.	LF		3
* -	**{Severity H}			

# **COMPONENTS (Continued)**

# **◆ 16.04.04 CONNECTIONS**

Joint and connections for steel beams, diaphragm and cross frame may be either welded, bolted, riveted, or pinned.

bolted, riveted, or pinned.			
Defect:	UOM	LEVEL II KEY	LEVEL III KEY
* Corrosion - Gussets or Connection Plates: Observation:			
<ul><li>a. Surface rust no pitting evident.</li><li>***{Severity L)</li></ul>	SF		
<ul> <li>b. Corrosion evident pitting and blistering of base material.</li> <li>***{Severity M)</li> </ul>	SF		
<ul><li>c. Corrosion evident with loss to base section.</li><li>***{Severity H}</li></ul>	SF		4
Defect:			
* Corrosion - Bolts or Welds: Observation:			
<ul><li>a. Surface rust no pitting evident.</li><li>***{Severity L)</li></ul>	EA		
b. Corrosion evident pitting and blistering of base material.  ***{Severity M)	EA		
<ul><li>c. Corrosion evident with loss to base section.</li><li>***{Severity H}</li></ul>	EA		4
Defect:			
* Connectors or Fasteners: Observation:			
a. Loose bolts or fasteners.  ***{Severity L}	EA		
b. Missing fasteners or connectors.  ***{Severity H}	EA		
c. Crack in weld.  ***{Severity H}	LF		4
d. Crack in connection plate.  ***{Severity H}	LF		4
• • •			

#### **COMPONENTS (Continued)**

#### **◆ 16.04.05 BEARINGS**

A bridge bearing is a superstructure element which provides an interface between the superstructure and the substructure. The three primary functions of a bearing are:

- 1. To transmit all the dead and live loads from the superstructure.
- 2. To permit longitudinal movement of the superstructure due to thermal expansion and contraction.
- 3. To allow rotation caused by dead and live loads deflection.

Bearings that do not allow for translation or movement of the superstructure are referred to as fixed bearings. Bearings that do allow for translation or movement of the superstructure are known as expansion bearings. A bridge bearing consists of four basic parts which includes, sole plate, bearing or bearing surfaces, masonry plate and anchorage. Various expansion bearing types have evolved out of the need to accommodate superstructure movement.

Potential defects which may be observed include, corrosion, rotation and excess movement.

Defect:	UOM	LEVEL II KEY	LEVEL III KEY
* Metal Bearing - Expansion or Fixed Bearing:			
Observation:			
<ul> <li>a. Surface rust no evidence</li> </ul>	EA		
of pitting, expansion bearing			
free to rotate.			
* * * {Severity L}			
<ul> <li>b. Dirt and debris accumulated around</li> </ul>	SF		
base of bearing.			
* * * {Severity L}			
<ul> <li>c. Corrosion pitting and</li> </ul>	EA ,		
Blistering of base metal, expansion			
bearing free to rotate.			
* * * {Severity M}			
<ul> <li>d. Corrosion with loss to base</li> </ul>	EA		5
section, expansion			
bearing frozen.			
* * * {Severity H}			
<ul> <li>e. Excess rotation of expansion bearing.</li> </ul>	EA		5
* * * {Severity H}			
f. Loss of bearing area due to lateral	EA		5
or longitudinal movement.			
* * * {Severity H}			

16 Bridge

# **COMPONENTS (Continued)**

# ◆ 16.04.05 BEARINGS (Continued)

Defect:	МОП	LEVEL II KEY	LEVEL III KEY
* Elastomeric Bearing:			
Observation:			
<ol> <li>Bulging of the bearing.</li> </ol>	EA		5
* * * {Severity M}			
<ul> <li>b. Splitting or tearing of the</li> </ul>	EA	•	5
bearing, including interior			
steel shims bond.			
* * * {Severity M}			
<ul> <li>c. Bond to the sole and or masonry</li> </ul>	EA		5
plate failed.			
* * * {Severity H}			
<ul> <li>d. Excess longitudinal movement.</li> </ul>	EA		5
* * * {Severity H}			
<ul> <li>e. Excess rotation movement.</li> </ul>	EA		5
* * * {Severity H}			
f. Loss of bearing area due to lateral	EA		5
or longitudinal movement.			
* * * {Severity H}			

# **Defect:**

# \* Pin and Link Bearing:

	_
Ohee	rvation:
CDSC	i vation.

a.	Surface rust no pitting evident,	EA
	joint free to rotate.	
* * *	Severity L)	
b.	Corrosion evident pitting and	EA
	blistering of base material.	
* * *	{Severity M)	

c. Corrosion evident with loss to base section, joint frozen.

\*\*\*{Severity H}

5

EΑ

# **COMPONENTS (Continued)**

# ◆ 16.04.05 BEARINGS (Continued)

Defect:	UOM	LEVEL II KEY	LEVEL III KEY
* Corrosion - Sole and Masonry Plates:			
Observation:			
<pre>a. Surface rust no pitting evident. ***{Severity L)</pre>	SF		
<ul><li>b. Corrosion evident pitting and blistering of base material.</li><li>***{Severity M}</li></ul>	SF		
c. Corrosion evident with loss to base section.	SF		5
***{Severity H}			
Defect:			
* Anchor Bolts:			
Observation:			

<ul> <li>a. Surface rust no pitting evident.</li> </ul>	EA	
* * * {Severity L}		
• • •		
<ul> <li>b. Loose anchor bolts, nuts or bearing.</li> </ul>	EA	
* * * {Severity L}		
c. Corrosion evident pitting and	EA	
	<del> \</del>	
blistering of base material.		
* * * {Severity M}		
d. Corrosion evident with loss to	EA	5
	LA	ວ
base section.		
* * * {Severity H}		
·		
e. Missing or broken anchor bolts.	EA	5
* * * {Severity H}		
foctority is		

#### REFERENCES

- 1. U.S. Department of transportation, Federal Highway Administration, Bureau of Public roads (DOT) Bridge Inspector's Training Manual, 1990 Edition.
- 2. Bridge inspection and Rehabilitation, Parsons Brinckerhoff Edited by L.G. Silano, PE, 1993
- 3. Inspection of Bridges and Trestles NAVFAC MO-126, October 1991
- ACI Manual of Concrete Practice 1989, Part 1 Materials and General Properties of Concrete
- 5. AASHTO Manual for Maintenance Inspection of Bridges, American Association of State Highway and Transportation Officials
- 6. Micro Bridger (Version 1.0), U.S. Army Construction Engineering Research Laboratory

LEVEL II KEY	GUIDE SHEET CONTROL NUMBER
1	GS-II 16.04.01-1
2	GS-II 16.04.02-2
LEVEL III KEY	GUIDE SHEET CONTROL NUMBER
1	CC III 16 04 01 1
1 2	GS-III 16.04.01-1 GS-III 16.04.02-2
3	GS-III 16.04.03-3
4	GS-III 16.04.04-4
5	GS-III 16.04.05-5

#### **LEVEL II GUIDE SHEET - KEY NO. 1**

COMPONENT:

TIMBER PRIMARY AND SECONDARY MEMBERS

**CONTROL NUMBER:** 

GS-II 16.04.01-1

#### **Application**

This applies to the investigation of possible interior and exterior deterioration of timber primary and secondary members due to decay (rot), or parasites.

The results of the Level II inspection can be used to trigger a level III inspection or necessary repair.

### **Special Safety Requirements**

No special Safety Requirements, beyond the requirements listed in the Safety Section, are to be observed in the performance of the Level II.

#### **Inspection Actions**

- 1. Clean affected area.
- 2. Measure affected area.
- 3. Tap with hammer to determine extend of hollow or sound material.
- 4. Probe with ice pick.

#### **Recommended inspection Frequency**

As triggered by Level I defect/observation, and where this Level II inspection is the standard inspection procedure.

#### References

- U.S. Department of transportation, Federal Highway Administration, Bureau of Public roads (DOT) Bridge Inspector's Training Manual, 1990 Edition
- 2. Bridge inspection and Rehabilitation, Parsons Brinckerhoff Edited by L.G. Silano, PE, 1993
- 3. Inspection of Bridges and Trestles NAVFAC MO-126, October 1991
- 5. AASHTO Manual for Maintenance Inspection of Bridges, American Association of State Highway and Transportation Officials
- 6. Micro Bridger (Version 1.0), U.S. Army Construction Engineering Research Laboratory

### **LEVEL II GUIDE SHEET - KEY NO. 2**

COMPONENT:

CONCRETE PRIMARY AND SECONDARY MEMBERS

**CONTROL NUMBER:** 

GS-II 16.04.02-2

### **Application**

This applies to the investigation of concrete deterioration to primary and secondary members due to spalling from delamination.

The results of the Level II inspection can be used to trigger a level III inspection or necessary repair.

### **Special Safety Requirements**

No special Safety Requirements, beyond the requirements listed in the Safety Section, are to be observed in the performance of the level II.

#### **Inspection Actions**

- 1. Clean loose concrete from area to be inspected.
- 2. Measure the affected area.
- 3. Tap the affected area with a hammer to determine extent of unsound or hollow concrete.

#### **Recommended Inspection Frequency**

As triggered by Level I defect/observation, and where this Level II inspection is the standard inspection procedure.

#### References

- 1. U.S. Department of transportation, Federal Highway Administration, Bureau of Public roads (DOT) Bridge Inspector's Training Manual, 1990 Edition
- 2. Bridge inspection and Rehabilitation, Parsons Brinckerhoff Edited by L.G. Silano, PE, 1993
- 3. Inspection of Bridges and Trestles NAVFAC MO-126, October 1991
- 4. ACI Manual of Concrete Practice 1989, Part 1 Materials and General Properties of Concrete
- 5. AASHTO Manual for Maintenance Inspection of Bridges, American Association of State Highway and Transportation Officials
- 6. Micro Bridger (Version 1.0), U.S. Army Construction Engineering Research Laboratory

#### LEVEL III GUIDE SHEET - KEY NO. 1

COMPONENT:

TIMBER PRIMARY AND SECONDARY MEMBERS

**CONTROL NUMBER:** 

GS-III 16.04.01-1

#### **Application**

This guide has been prepared to identify the purpose of a Level III inspection and its more sophisticated test and inspection methods which may be appropriate to determine the cause and/or extent of defects recorded in Level I or Level II defect observations at the timber primary and secondary members.

Whereas the purpose of the Level I inspection was to record the observable defects on the primary and secondary members, this Level III inspection is performed to provide a thorough systematic evaluation of the observed defect and to make an assessment of it's effects, if left unchecked, on the safety, durability and stability of the bridge and it's appurtenant works.

The Level III inspection should be performed when prompted by the results of a Level I or II inspection. The inspection should be performed by an engineer or multidisciplined team of engineers experienced in the design and construction of bridges.

The results of the Level III inspection will be used to develop maintenance or remedial measure work strategy that will correct the existing deficiency conditions or to require continued monitoring of existing deficiency conditions on the bridge superstructure.

In general, appropriate advanced inspection methods will be identified, recommended, and performed by or under the supervision of the inspection engineer personnel as part of the Level III test and inspection method. Advanced inspection methods will be assigned only after the assessment of defect conditions observed during a Level I or II inspection.

### **Special Safety Requirements**

Passing traffic is a hazard. Level III inspection and testing must be performed with the prior approval of the Facility Manager who will notify the authorities to provide safety measures and safe access.

# LEVEL III GUIDE SHEET - KEY NO. 1 (Continued)

COMPONENT:

TIMBER PRIMARY AND SECONDARY MEMBERS

**CONTROL NUMBER:** 

GS-III 16.04.01-1

#### **Inspection Action**

1. Prior to making a field inspection of the observed defect, review all past records concerning the timber primary and secondary members the defective component if available. These records may include pre-construction investigation records, design criteria and analysis records, available construction records, previous periodic maintenance inspection records, water level records, and photographs taken during initial construction and subsequent inspections.

- 2. Perform inspection of the pertinent components where observed defects that triggered a Level III inspection are listed.
- 3. Make an assessment of the importance of individual defects observed for a given component at the bridge site. Indicate priorities for any required maintenance, or remedial measure work.
- 4. Identify whether particular observed defects need additional or continued observation.
- 5. Assess the stability and safety of the bridge superstructure.
- 6. Prepare final cost estimate for advanced inspection methods required to determine the cause and extent of the observed defect.
- 7. Prepare cost estimate for required maintenance or remedial repair measures, as applicable.

Level III advanced inspection methods may be required for specific Level I and Level II defect conditions observed at a bridge site. Level III advanced test or inspection methods and associated observed defects for primary and secondary members include, but are not limited to the following:

DOD CAS Manual 16 Bridges

#### LEVEL III INSPECTION METHOD GUIDE SHEET

### **LEVEL III GUIDE SHEET - KEY NO. 1 (Continued)**

COMPONENT:

TIMBER PRIMARY AND SECONDARY MEMBERS

**CONTROL NUMBER:** 

GS-III 16.04.01-1

#### Advanced Test or Inspection Method Applicable Observed Defects

1. Increment borer interior and exterior deterioration of timber due to

decay or parasites

2. Moisture content deterioration due to decay or parasites

3. Ultrasonic testing splits and internal flaws

4. Survey measurements members out-of-alignment

#### **Special Instructions**

Review as-built and design drawings of structure.

### **Special Tools & Equipment Requirements**

Increment borer
Survey level and rod
Navigable boat with related safety equipment
Moisture meter
Ultrasonic testing

#### **Recommended Inspection Frequency**

As triggered by Level I and II defect/observations or every 3 years.

#### References

- 1. U.S. Department of transportation, Federal Highway Administration, Bureau of Public roads (DOT) Bridge Inspector's Training Manual, 1990 Edition
- 2. Bridge inspection and Rehabilitation, Parsons Brinckerhoff Edited by L.G. Silano, PE, 1993
- Inspection of Bridges and Trestles NAVFAC MO-126, October 1991
- 4. ACI Manual of Concrete Practice 1989, Part 1 Materials and General Properties of Concrete
- 5. AASHTO Manual for Maintenance Inspection of Bridges, American Association of State Highway and Transportation Officials
- 6. Micro Bridger (Version 1.0), U.S. Army Construction Engineering Research Laboratory

#### **LEVEL III GUIDE SHEET - KEY NO. 2**

COMPONENT:

CONCRETE PRIMARY AND SECONDARY MEMBERS

**CONTROL NUMBER:** 

GS-III 16.04.02-2

### **Application**

This guide has been prepared to identify the purpose of a Level III inspection and its more sophisticated test and inspection methods which may be appropriate to determine the cause and/or extent of defects recorded in Level I or Level II defect observations at the concrete primary and secondary members.

Whereas the purpose of the Level I inspection was to record the observable defects on the primary and secondary members, this Level III inspection is performed to provide a thorough systematic evaluation of the observed defect and to make an assessment of it's effects, if left unchecked, on the safety, durability and stability of the primary and secondary members and it's appurtenant works.

The Level III inspection should be performed when prompted by the results of a Level I or II inspection. The inspection should be performed by an engineer or multidisciplined team of engineers experienced in the design and construction of bridges.

The results of the Level III inspection will be used to develop maintenance or remedial measure work strategy that will correct the existing deficiency conditions or to require continued monitoring of existing deficiency conditions on the bridge superstructure.

In general, appropriate advanced inspection methods will be identified, recommended, and performed by or under the supervision of the inspection engineer personnel as part of the Level III test and inspection method. Advanced inspection methods will be assigned only after the assessment of defect conditions observed during a Level I or II inspection.

# **Special Safety Requirements**

Passing traffic is a hazard. Level III inspection and testing must be performed with the prior approval of the Facility Manager who will notify the authorities to provide safety measures and safe access.

DOD CAS Manual 16 Bridges

#### LEVEL III INSPECTION METHOD GUIDE SHEET

#### **LEVEL III GUIDE SHEET - KEY NO. 2 (Continued)**

COMPONENT:

CONCRETE PRIMARY AND SECONDARY MEMBERS

**CONTROL NUMBER:** 

GS-III 16.04.02-2

#### **Inspection Action**

1. Prior to making a field inspection of the observed defect, review all past records concerning the primary and secondary members and the defective component if available. These records may include pre-construction investigation records, design criteria and analysis records, available construction records, previous periodic maintenance inspection records, water level records, and photographs taken during initial construction and subsequent inspections.

- 2. Perform inspection of the pertinent components where observed defects that triggered a Level III inspection are listed.
- 3. Make an assessment of the importance of individual defects observed for a given component at the bridge site. Indicate priorities for any required maintenance, or remedial measure work.
- 4. Identify whether particular observed defects need additional or continued observation.
- 5. Assess the stability and safety of the bridge superstructure.
- 6. Prepare final cost estimate for advanced inspection methods required to determine the cause and extent of the observed defect.
- 7. Prepare cost estimate for required maintenance or remedial repair measures, as applicable.

Level III advanced inspection methods may be required for specific Level I and Level II defect conditions observed at a bridge site. Level III advanced test or inspection methods and associated observed defects for primary and secondary members include, but are not limited to the following:

# LEVEL III GUIDE SHEET - KEY NO. 2 (Continued)

COMPONENT:

CONCRETE PRIMARY AND SECONDARY MEMBERS

**CONTROL NUMBER:** 

GS-III 16.04.02-2

#### 

1. Infrared Thermography and ground probing radar

concrete spalling and delamination

2. Concrete coring

concrete deterioration

3. Laboratory test on concrete (core, strength tests, abrasion, absorption, sulfate soundness, unit weight

concrete deterioration and strength

4. Ultrasonic test

internal cracks and spalling, delamination

5. Half-cell test

corrosion to reinforcement steel

6. Survey measurements

member out-of-alignment

#### **Special Instructions**

Review as-built and design drawings of structure.

#### **Special Tools & Equipment Requirements**

Industry required testing equipment needed to perform the advanced investigation method chosen.

Navigable boat with related safety equipment.

Surveying equipment

# **Recommended Inspection Frequency**

As triggered by Level I and II defect/observations or every 3 years.

# LEVEL III GUIDE SHEET - KEY NO. 2 (Continued)

COMPONENT:

CONCRETE PRIMARY AND SECONDARY MEMBERS

**CONTROL NUMBER:** 

GS-III 16.04.02-2

#### References

1. U.S. Department of transportation, Federal Highway Administration, Bureau of Public roads (DOT) Bridge Inspector's Training Manual, 1990 Edition

- 2. Bridge inspection and Rehabilitation, Parsons Brinckerhoff Edited by L.G. Silano, PE, 1993
- 3. Inspection of Bridges and Trestles NAVFAC MO-126, October 1991
- 4. ACI Manual of Concrete Practice 1989, Part 1 Materials and General Properties of Concrete
- 5. AASHTO Manual for Maintenance Inspection of Bridges, American Association of State Highway and Transportation Officials
- 6. Micro Bridger (Version 1.0), U.S. Army Construction Engineering Research Laboratory

#### LEVEL III GUIDE SHEET - KEY NO. 3

COMPONENT:

STEEL PRIMARY AND SECONDARY MEMBERS

**CONTROL NUMBER:** 

GS-III 16.04.03-3

#### **Application**

This guide has been prepared to identify the purpose of a Level III inspection and its more sophisticated test and inspection methods which may be appropriate to determine the cause and/or extent of defects recorded in Level I or Level II defect observations at the steel primary and secondary members.

Whereas the purpose of the Level I inspection was to record the observable defects at the primary and secondary members, this Level III inspection is performed to provide a thorough systematic evaluation of the observed defect and to make an assessment of it's effects, if left unchecked, on the safety, durability and stability of the primary and secondary member and it's appurtenant works.

The Level III inspection should be performed when prompted by the results of a Level I or II inspection. The inspection should be performed by an engineer or multidisciplined team of engineers experienced in the design and construction of bridges.

The results of the Level III inspection will be used to develop maintenance or remedial measure work strategy that will correct the existing deficiency conditions or to require continued monitoring of existing deficiency conditions on the primary and secondary members.

In general, appropriate advanced inspection methods will be identified, recommended, and performed by or under the supervision of the inspection engineer personnel as part of the Level III test and inspection method. Advanced inspection methods will be assigned only after the assessment of defect conditions observed during a Level I or II inspection.

# **Special Safety Requirements**

Passing traffic is a hazard. Level III inspection and testing must be performed with the prior approval of the Facility Manager who will notify the authorities to provide safety measures and safe access.

#### LEVEL III GUIDE SHEET - KEY NO. 3 (Continued)

COMPONENT:

STEEL PRIMARY AND SECONDARY MEMBERS

CONTROL NUMBER:

GS-III 16.04.03-3

### **Inspection Action**

- Prior to making a field inspection of the observed defect, review all past records concerning the primary and secondary members and the defective component if available. These records may include pre-construction investigation records, design criteria and analysis records, available construction records, previous periodic maintenance inspection records, water level records, and photographs taken during initial construction and subsequent inspections.
- 2. Perform inspection of the pertinent components where observed defects that triggered a Level III inspection are listed.
- Make an assessment of the importance of individual defects observed for a given component at the bridge site. Indicate priorities for any required maintenance, or remedial measure work.
- 4. Identify whether particular observed defects need additional or continued observation.
- 5. Assess the stability and safety of the bridge superstructure.
- 6. Prepare final cost estimate for advanced inspection methods required to determine the cause and extent of the observed defect.
- 7. Prepare cost estimate for required maintenance or remedial repair measures, as applicable.

Level III advanced inspection methods may be required for specific Level I and Level II defect conditions observed at a bridge site. Level III advanced test or inspection methods and associated observed defects for primary and secondary members include, but are not limited to the following:

# LEVEL III GUIDE SHEET - KEY NO. 3 (Continued)

COMPONENT:

STEEL PRIMARY AND SECONDARY MEMBER

**CONTROL NUMBER:** 

GS-III 16.04.03-3

#### 

1. Grinding and or sandblasting, using caliper to measure

corrosion of steel and section loss

section loss

2. Magnetic particle

cracks in steel or welds

3. Dye-Penetrant

cracks in steel or welds

3. Ultrasonic test

cracks and voids in steel

4. Survey measurements

member out-of-alignment

#### **Special Instructions**

Review as-built and design drawings of structure.

#### **Special Tools & Equipment Requirements**

Grinder or sandblasting equipment

Surveying equipment

Navigable boat with related safety equipment

Industry required testing equipment needed to perform the advanced investigation method chosen

#### Recommended Inspection Frequency

As triggered by Level I and II defect/observations or every 3 years.

#### References

- U.S. Department of transportation, Federal Highway Administration, Bureau of Public roads (DOT) Bridge Inspector's Training Manual, 1990 Edition
- 2. Bridge inspection and Rehabilitation, Parsons Brinckerhoff Edited by L.G. Silano, PE, 1993
- 3. Inspection of Bridges and Trestles NAVFAC MO-126, October 1991
- 4. AASHTO Manual for Maintenance Inspection of Bridges, American Association of State Highway and Transportation Officials
- 5. Micro Bridger (Version 1.0), U.S. Army Construction Engineering Research Laboratory

### LEVEL III GUIDE SHEET - KEY NO. 4

COMPONENT:

**CONNECTIONS** 

**CONTROL NUMBER:** 

GS-III 16.04.04-4

### **Application**

This guide has been prepared to identify the purpose of a Level III inspection and its more sophisticated test and inspection methods which may be appropriate to determine the cause and/or extent of defects recorded in Level I or Level II defect observations at the connection.

Whereas the purpose of the Level I inspection was to record the observable defects at the connections, this Level III inspection is performed to provide a thorough systematic evaluation of the observed defect and to make an assessment of it's effects, if left unchecked, on the safety, durability and stability of the superstructural and it's appurtenant works.

The Level III inspection should be performed when prompted by the results of a Level I or II inspection. The inspection should be performed by an engineer or multidisciplined team of engineers experienced in the design and construction of bridges.

The results of the Level III inspection will be used to develop maintenance or remedial measure work strategy that will correct the existing deficiency conditions or to require continued monitoring of existing deficiency conditions on the bridges.

In general, appropriate advanced inspection methods will be identified, recommended, and performed by or under the supervision of the inspection engineer personnel as part of the Level III test and inspection method. Advanced inspection methods will be assigned only after the assessment of defect conditions observed during a Level I or II inspection.

# **Special Safety Requirements**

Passing traffic is a hazard. Level III inspection and testing must be performed with the prior approval of the Facility Manager who will notify the authorities to provide safety measures and safe access.

### LEVEL III GUIDE SHEET - KEY NO. 4 (Continued)

COMPONENT: CONTROL NUMBER: CONNECTIONS

GS-III 16.04.04-4

# **Inspection Action**

- 1. Prior to making a field inspection of the observed defect, review all past records concerning the connections and the defective component if available. These records may include pre-construction investigation records, design criteria and analysis records, available construction records, previous periodic maintenance inspection records, water level records, and photographs taken during initial construction and subsequent inspections.
- 2. Perform inspection of the pertinent components where observed defects that triggered a Level III inspection are listed.
- 3. Make an assessment of the importance of individual defects observed for a given component at the bridge site. Indicate priorities for any required maintenance, or remedial measure work.
- 4. Identify whether particular observed defects need additional or continued observation.
- 5. Assess the stability and safety of the bridge superstructure.
- 6. Prepare final cost estimate for advanced inspection methods required to determine the cause and extent of the observed defect.
- 7. Prepare cost estimate for required maintenance or remedial repair measures, as applicable.

Level III advanced inspection methods may be required for specific Level I and Level II defect conditions observed at a bridge site. Level III advanced test or inspection methods and associated observed defects for bridge superstructure include, but are not limited to the following:

# LEVEL III GUIDE SHEET - KEY NO. 4 (Continued)

COMPONENT:

**CONNECTIONS** 

**CONTROL NUMBER:** 

GS-III 16.04.04-4

#### 

1. Grinding and or sandblasting, using caliper to measure

corrosion of steel and section loss

section loss

2. Magnetic particle

cracks in steel or welds

3. Dye-Penetrant

cracks in steel or welds

3. Ultrasonic test

cracks and voids in steel

### **Special Instructions**

Review as-built and design drawings of structure.

### **Special Tools & Equipment Requirements**

Grinder or sandblaster equipment

Surveying equipment

Navigable boat with related safety equipment

Industry required testing equipment needed to perform the advanced investigation method chosen

#### Recommended Inspection Frequency

As triggered by Level I and II defect/observations or every 3 years.

### References

- 1. U.S. Department of transportation, Federal Highway Administration, Bureau of Public roads (DOT) Bridge Inspector's Training Manual, 1990 Edition
- 2. Bridge inspection and Rehabilitation, Parsons Brinckerhoff Edited by L.G. Silano, PE, 1993
- 3. Inspection of Bridges and Trestles NAVFAC MO-126, October 1991
- 4. AASHTO Manual for Maintenance Inspection of Bridges, American Association of State Highway and Transportation Officials
- 5. Micro Bridger (Version 1.0), U.S. Army Construction Engineering Research Laboratory

#### LEVEL III GUIDE SHEET - KEY NO. 5

COMPONENT:

BEARINGS

**CONTROL NUMBER:** 

GS-III 16.04.05-5

#### **Application**

This guide has been prepared to identify the purpose of a Level III inspection and its more sophisticated test and inspection methods which may be appropriate to determine the cause and/or extent of defects recorded in Level I or Level II defect observations at the bearings.

Whereas the purpose of the Level I inspection was to record the observable defects at the expansion joint, this Level III inspection is performed to provide a thorough systematic evaluation of the observed defect and to make an assessment of it's effects, if left unchecked, on the safety, durability and stability of the superstructural and it's appurtenant works.

The Level III inspection should be performed when prompted by the results of a Level I or II inspection. The inspection should be performed by an engineer or multidisciplined team of engineers experienced in the design and construction of bridges.

The results of the Level III inspection will be used to develop maintenance or remedial measure work strategy that will correct the existing deficiency conditions or to require continued monitoring of existing deficiency conditions on the bridge superstructure.

In general, appropriate advanced inspection methods will be identified, recommended, and performed by or under the supervision of the inspection engineer personnel as part of the Level III test and inspection method. Advanced inspection methods will be assigned only after the assessment of defect conditions observed during a Level I or II inspection.

# **Special Safety Requirements**

Passing traffic is a hazard. Level III inspection and testing must be performed with the prior approval of the Facility Manager who will notify the authorities to provide safety measures and safe access.

#### LEVEL III GUIDE SHEET - KEY NO. 5 (Continued)

COMPONENT:

**BEARINGS** 

**CONTROL NUMBER:** 

GS-III 16.04.05-5

### **Inspection Action**

- Prior to making a field inspection of the observed defect, review all past records concerning the bearings and the defective component if available. These records may include pre-construction investigation records, design criteria and analysis records, available construction records, previous periodic maintenance inspection records, water level records, and photographs taken during initial construction and subsequent inspections.
- 2. Perform inspection of the pertinent components where observed defects that triggered a Level III inspection are listed.
- 3. Make an assessment of the importance of individual defects observed for a given component at the bridge site. Indicate priorities for any required maintenance, or remedial measure work.
- 4. Identify whether particular observed defects need additional or continued observation.
- 5. Assess the stability and safety of the bearings.
- 6. Prepare final cost estimate for advanced inspection methods required to determine the cause and extent of the observed defect.
- 7. Prepare cost estimate for required maintenance or remedial repair measures, as applicable.

Level III advanced inspection methods may be required for specific Level I and Level II defect conditions observed at a bridge site. Level III advanced test or inspection methods and associated observed defects for the bearings include, but are not limited to the following:

# LEVEL III GUIDE SHEET - KEY NO. 5 (Continued)

**COMPONENT:** 

**BEARINGS** 

**CONTROL NUMBER:** 

GS-III 16.04.05-5

#### 

1. Grinding or sandblasting, using caliper to measure section loss

corrosion of steel and section loss

2. Magnetic particle

cracks in steel or welds

4. Dye-Penetrant

cracks in steel or welds

5. Ultrasonic test

cracks and voids in steel

6. Survey measurements

bearing movement

# **Special Instructions**

Review as-built and design drawings of structure.

# **Special Tools & Equipment Requirements**

Grinder or sandblasting equipment

Surveying equipment

navigable boat with related safety equipment

Industry required testing equipment needed to perform the advanced investigation method chosen

#### Recommended Inspection Frequency

As triggered by Level I and II defect/observations or every 3 years.

#### References

- 1. U.S. Department of Transportation, Federal Highway Administration, Bureau of Public roads (DOT) Bridge Inspector's Training Manual, 1990 Edition
- 2. Bridge Inspection and Rehabilitation, Parsons Brinckerhoff Edited by L.G. Silano, PE, 1993
- 3. Inspection of Bridges and Trestles NAVFAC MO-126, October 1991
- 4. AASHTO Manual for Maintenance Inspection of Bridges, American Association of State Highway and Transportation Officials
- 5. Micro Bridger (Version 1.0), U.S. Army Construction Engineering Research Laboratory

### 16.05 BRIDGE DECKS

#### DESCRIPTION

The primary function of a bridge deck is to provide a roadway over or through the superstructure which traffic can move and distribute the live and dead loads to the superstructure. There are three common materials used; Timber, Concrete and Steel. The specific structural function of a deck is determined by whether the deck is composite or noncomposite.

A composite deck is designed to join together the deck and supporting members such that they structurally behave as one member. A composite deck spans between its supports but also functions to increase the superstructure strength and allowable span length. Composite decks are often used in design, the most common application being the attachment of a deck to steel beams or girders.

A non-composite deck does not contribute to the structural capacity of the supporting members. A non-composite deck only functions to span between supporting members and to provide a wearing surface for the traffic.

Curb barriers which parallel the side limits of the bridge deck guide the movement of vehicle wheels and safeguard bridge trusses, railing or other construction existing outside the roadway limit. Pedestrian traffic on sidewalks are also protected from collision with vehicular traffic.

Some bridge decks provide deck area to serve pedestrian traffic only and, for safety and convenience to its users, these walkways are commonly elevated above the deck portion used by vehicles.

#### SPECIAL TOOL AND EQUIPMENT REQUIREMENTS

The following list of special tools and equipment, beyond the requirements listed in the standard Tool Section, may be required to preform the inspection of this subsystem:

- 1. Boat
- 2. Related safety equipment

#### SPECIAL SAFETY REQUIREMENTS

Level I and Level II inspection are performed by walking the bridge deck, and observing from ground level, thus passing traffic is a hazard. The inspection must be performed with prior approval of the Facility Manager, who will notify the necessary authorities so as to provide traffic safety measures and access. It is suggested that the inspector wear an orange safety vest.

Depending on the bridge type a boat may be required to observe certain components under the bridge deck. The inspector will take all necessary safety measures, and refer to the Master Safety Plan for guidance and compliance.

### COMPONENT LIST

- ◆ 16.05.01 ASPHALTIC WEARING SURFACE
- ◆ 16.05.02 TIMBER DECK
- ◆ 16.05.03 CONCRETE DECK
- ♦ 16.05.04 STEEL DECK
- ◆ 16.05.05 EXPANSION JOINTS

### RELATED SUBSYSTEMS

Due to the related nature of the elements requiring inspecting, the following DS/IMs should be reviewed for concurred inspection activities.

19.01	ROADWAYS
20.01	RAILROAD
21.00	WATERFRONT (all subsystems)
29.00	SITE ELECTRICAL (all subsystems)

#### STANDARD INSPECTION PROCEDURE

The Level I inspection shall be carried out for each component in the order listed. The inspector will identify any physical defects existing in each component; will note the level of severity of each defect type; and measure the quantity of each defect. Both the top surface and underside shall be inspected.

DOD CAS Manual 16 Bridge

### **16.05 BRIDGE DECKS**

#### COMPONENTS

### **◆ 16.05.01 ASPHALTIC WEARING SURFACE**

The wearing surface provides the riding surface for traffic and is placed on top of the structural deck. Wearing surfaces are either asphaltic concrete, portland cement or timber. A timber wearing surface may consist of longitudinal timber placed over the transverse decking. Timber wearing surface is often referred to as "Runner Boards" and are placed longitudinally only in the strip where the wheels of vehicles ride. When the wearing surface is concrete and poured simultaneously with the slab it is referred to as a monolithic deck.

Defect:	UOM	KEY KEY
* Alligator or Fatigue Cracking: When two or three levels of severity exist within one distressed area and if these can be easily distinguished from each other, they should be measured and recorded separately. However, if the different levels of severity cannot be easily divided, the entire area should be rated at the highest severity level present.  Observation:		
a. Longitudinal disconnected hairline cracks running parallel to each other. The cracks are not spalled. Initially there may only be a single crack in the wheel path.  * * * {Severity L}	SF	1
b. Further development of low severity alligator cracking into a pattern of pieces formed by cracks that may be lightly surface-spalled.  * * * {Severity M}	SF	1
c. Medium alligator cracking has progressed so that pieces are more severely spalled at the edges and loosened until the cells rock under traffic. Pumping may also exist.	SF	1

LEVEL II LEVEL III

\* \* \* {Severity H}

### **COMPONENTS (Continued)**

◆ 16.05.01 ASPHALTIC WEARING SURFACE (Continued)

Defect: LEVEL III LEVEL III

Defect: UOM KEY KEY

### \* Asphalt Bleeding:

Observation:

- a. Bleeding has only occurred to a very slight degree and it is noticeable only during a few days a year. Asphalt does not stick to shoes or vehicles.
- \*\*\* {Severity L}
- Bleeding has occurred to the extent SF that asphalt sticks to shoes and vehicles during only a few weeks of the year.
- \*\*\* {Severity M}
- c. Bleeding has occurred extensively and SF considerable asphalt sticks to shoes and vehicles during at least several weeks of the year.
- \* \* \* {Severity H}

### **COMPONENTS (Continued)**

◆ 16.05.01 A	SPHALTIC WEARING SURFACE (Conti	nued)		
Defect:		иом	KEY	KEY
* Block (	Cracking			
Obs	servation:			
a.	Blocks are defined by non-sealed cracks that are non-spalled (sides of the crack are vertical) or only minor spalling with a ¼-inch or less mean width.	SF		
* * *	*{Severity L}			
b.	Blocks are defined by sealed cracks that have a sealant in satisfactory condition to prevent moisture infiltration.	SF		
* * *	*{Severity L}			
C.	Blocks are defined by sealed or non-sealed cracks that are moderately spalled.	SF		1
* * *	*{Severity M}			
d.	Blocks are defined by non-sealed cracks that are not spalled or have only minor spalling, but have a mean width greater than	SF		

SF

SF

\* \* \* {Severity M}

\* \* \* {Severity M}

\*\*\* {Severity H}

approximately 1/4-inch.

Blocks are defined by sealed

cracks that are not spalled or have only minor spalling, but have sealant in unsatisfactory condition to prevent moisture infiltration.

Blocks are well defined by cracks

that are severely spalled.

## **COMPONENTS (Continued)**

\*\*\*{Severity H}

### ◆ 16.05.01 ASPHALTIC WEARING SURFACE (Continued)

Defect:	UOM	LEVEL II KEY	LEVEL III KEY
* Edge Cracking:			
Observation:			
<ul> <li>a. Low or medium severity cracking with no breakup or raveling.</li> </ul>	SF		
* * * {Severity L}			
b. Medium severity cracks with some breakup or raveling.	SF		
* * * {Severity M}			
<ul> <li>Considerable breakup or raveling along the edge. Broken pieces may be removable.</li> </ul>	SF e		1

Defect:

# 16.05 BRIDGE DECKS

# **COMPONENTS (Continued)**

# ♦ 16.05.01 ASPHALTIC WEARING SURFACE (Continued)

1	:: :	UOM	LEVEL II KEY	LEVEL III KEY
	* Longitudinal and Transverse Cracking: The vehicle used to determine bump severity is a mid- to full-size sedan weighing approximately 3,000 to 3,800 lb. over the pavement inspection unit at the posted speed limit.  Observation:			
	a. Non-sealed cracks have either minor spalling or no spalling; the cracks have a mean width of ¼-inch or less.  *** {Severity L}	LF		
	<ul> <li>Sealed cracks have either minor spalling or no spalling; cracks are of any width, but their sealant material is in satisfactory condition to substantially prevent water infiltration.</li> </ul>	LF		
	*** {Severity L}  c. No significant bump occurs when a vehicle crosses the crack.	LF		
	*** {Severity L} d. Cracks are moderately spalled and can be either sealed or non-sealed of any width.  *** {Severity M}	LF		
	e. Sealed cracks are not spalled or have only minor spalling, but the sealant is in a condition so that water can freely infiltrate.  *** {Severity M}	LF		
	f. Non-sealed cracks are not spalled or are only lightly spalled, but the mean crack width is greater than 1/4-inch.	LF		
	<pre>*** {Severity M} g. Low-severity random cracking exists     near the crack or at the corners of     intersecting cracks. *** {Severity M}</pre>	LF		

### **COMPONENTS (Continued)**

### ◆ 16.05.01 ASPHALTIC WEARING SURFACE (Continued)

Defect:	UOM	LEVEL II	KEY
* Longitudinal and Transverse Cracking (Con	tinued)		
<ul><li>h. The crack causes a significant bur</li><li>to a vehicle.</li><li>***{Severity M}</li></ul>	np LF		
<ul> <li>i. Cracks are severely spalled and/or there exists medium or high rando cracking near the crack or at the corners of intersecting cracks.</li> </ul>			1
* * * {Severity H}			
<ul> <li>j. The crack causes a severe bump to vehicle.</li> </ul>	oa LF		1
* * * {Severity H}			

### Defect:

### \* Patch Deterioration:

The vehicle used to determine patch condition severity is a mid- to full-size sedan weighing approximately 3,000 to 3,800 lb. over the pavement inspection unit at the posted speed limit.

### Observation:

a.	Patch is in very good condition	SF
	and is performing satisfactorily.	
* * *	{Severity L}	
b.	Patch is somewhat deteriorated,	SF
	having low to medium levels of	
	any types of distress.	
* * *	{Severity M}	
c.	The patch causes a significant	SF
	bump to a vehicle.	
* * *	{Severity M}	
d.	Patch is badly deteriorated and	SF
	soon needs replacement.	
* * *	{Severity H}	
e.	The patch causes a severe bump	SF
	to a vehicle.	
* * *	{Severity H}	

1

### **16.05 BRIDGE DECKS**

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VVII	/II \	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	$\mathbf{I} \cup \mathbf{I} \cup \mathbf{I}$	/! ! L!! ! U	Cui

### ◆ 16.05.01 ASPHALTIC WEARING SURFACE (Continued)

LEVEL II LEVEL III
Defect: UOM KEY KEY

SF

SF

SF

### \* Polished Aggregate:

The existence of polishing can be detected by both visually observing and running the fingers over the surface.

#### Observation:

- Aggregate extending above the pavement is negligible, and the surface aggregate is smooth to the touch.
- \* \* \* {Severity L}
- b. Pavement surface is smooth and has a distinctive dull finish.
- \* \* \* {Severity M}
- Pavement surface appears highly smooth and the aggregate are highly polished.
- \* \* \* {Severity H}

#### Defect:

#### \* Potholes:

#### Observation:

- a. Pothole area up to 3 SF and depth SF less than 1 inch.
- \* \* \* {Severity L}
- b. Pothole area up to 3 SF and depth SF between 1 and 2 inches.
- \*\*\*{Severity M}
- c. Pothole area more than 3 SF and SF depth less than 1 inch.
- \* \* \* {Severity M}
- d. Pothole area less than 1 SF and SF depth more than 2 inches.
- \*\*\*{Severity M}
- e. Pothole area between 1 and 3 SF SF and depth more than 2 inches.
- \*\*\*{Severity H}

**Defect:** 

# **16.05 BRIDGE DECKS**

# **COMPONENTS (Continued)**

# ◆ 16.05.01 ASPHALTIC WEARING SURFACE (Continued)

t:	иом	LEVEL II KEY	LEVEL III KEY
* Reflection Cracking The vehicle used to determine bump is a mid to full-size sedan weighing approximately 3,000 to 3,800 lbs. over the pavement inspection unit at the posted speed limit.			
Observation:  a. Cracks have either minor spalling or no spalling. Non-sealed cracks have a mean width of ¼-inch or less.	LF		
*** {Severity L} b. Cracks have either minor spalling or no spalling. Cracks are sealed and of any width, both their sealant material is in satisfactory condition to substantially prevent water infiltration.	LF		
* * * {Severity L}  c. No significant bump occurs when a vehicle crosses the crack.  * * * * {Severity L}	LF		
* * * {Severity L} d. Cracks are moderately spalled and can be either sealed or non-sealed of any width.	LF		
<ul> <li>***{Severity M}</li> <li>e. Sealed cracks are not spalled or have only minor spalling, but the sealant is in a condition so that water can freely infiltrate.</li> <li>***{Severity M}</li> </ul>	LF		
f. Non-sealed cracks are not spalled or are only lightly spalled, but the mean crack width is greater than ¼-inch.  ***{Severity M}	LF		
g. Low-severity random cracking exists near the crack or at the corners of intersecting cracks.  * * * {Severity M}	LF		

# **COMPONENTS (Continued)**

### **◆** 16.05.01 ASPHALTIC WEARING SURFACE (Continued)

Defect:	иом	LEVEL II KEY	KEY
* Reflection Cracking (Continued)			
<ul> <li>h. The crack causes a significant bump to a vehicle.</li> </ul>	b LF		
* * * {Severity M}			
<ul> <li>i. Cracks are severely spalled and/or there exists medium or high random cracking near the crack or a the corners of intersecting cracks.</li> </ul>	LF at		
* * * {Severity H}			
<ul> <li>j. The crack causes a severe bump to a vehicle.</li> </ul>	LF		
* * * {Severity H}			

### Defect:

### \* Rutting:

Rutting severity is determined by the mean depth of the rut. To determine the mean depth, a 4-foot straight edge should be laid across the rut and the maximum depth measured. The mean depth should be computed from measurements taken every 20 feet along the length of the rut.

#### Observation:

SF	
SF	1
SF	1
	SF

### **COMPONENTS**

### ◆ 16.05.02 TIMBER DECK

The primary function of a bridge deck is to provide a roadway over which traffic can move and to distribute live and dead loads to the superstructure. On some bridge decks sidewalk and curbs are provided and are considered part of the bridge deck. The three most common materials used in construction of bridge decks are timber, concrete and steel deck.

The potential defect which may be observed in timber decks include decay, parasites, deterioration, weathering and overloads.

Defect:	UOM	LEVEL II KEY	LEVEL III KEY
* Weathering and Wear:			
Observation:			
<ul> <li>a. Surface of wood is rough</li> <li>and corrugated and member</li> <li>may be warped.</li> </ul>	SF		
* * * {Severity L}			
b. Surface of wood is rough and corrugated with cracks partially through the wood member, may have minor section loss to the top surface.  Member may be warped.	SF	1	
* * * {Severity M} c. Large cracks extend deeply or completely through the wood.  * * * {Severity H}	SF		2
<pre>***{Severity H} d. Wood is crumbly and seriously</pre>	SF		2

# **COMPONENTS**

### ◆ 16.05.02 TIMBER DECK (Continued)

Defect:	иом	LEVEL II KEY	LEVEL III
* Deflection:			
Observation:  a. Slight deflection of member when vehicles pass.  * * * {Severity L}	LF		
b. Noticeable deflection of member when vehicles pass.  * * * {Severity M}	LF		
c. Large deflection of member when vehicles pass.  ***{Severity H}	LF		2
d. Permanent deformation of member ***{Severity H}	. LF		2
Defect:			
* Vibration - Per Span:			
Observation:  a. Slight vibration in deck when  vehicles pass.  * * * {Severity L}	LF		
b. Noticeable vibration in deck when vehicles pass.  ***{Severity M}	LF		
c. Excessive vibration in deck when vehicles pass.  ***{Severity H}	LF		2
Defect:		•	
* Vehicular Damage: Observation:			
<ul><li>a. Member out of alignment.</li><li>***{Severity H}</li></ul>	LF		3
<ul><li>b. Shattered or injured timber member.</li><li>***{Severity H}</li></ul>	LF		3

## **COMPONENTS (Continued)**

### ◆ 16.05.02 TIMBER DECK (Continued)

		LEVEL II	LEVEL III
Defect:	UOM	KEY	KEY

SF

SF

EΑ

### \* Decay (Rot/Fungus Decay):

Decay from rot/fungus is most likely to occur at connections, splices, support points or around bolt holes. This may be due either to the tendency of such areas to collect and retain moisture, or to bolt holes or cuts being made in the surface after the preservative treatment has been applied.

### Observation:

- Moist and stain or discolored area signs of fungi, surface is solid.
- \* \* \* {Severity L}
- b. Surface spongy, member may SF show signs of crushing.
- \* \* \* {Severity M}
- c. Brown and white discolored area, member may show section loss and crushing.
- \* \* \* {Severity H}

### Defect:

#### \* Connections:

### Observation:

- a. Loose fasteners.
- \* \* \* {Severity M}
- b. Member broken, split or damaged. EA
- \* \* \* {Severity H}
- c. Missing fasteners or anchorage. EA
- \*\*\*{Severity H}

2

# **COMPONENTS (Continued)**

# ◆ 16.05.02 TIMBER DECK (Continued)

Defect:	иом	LEVEL II KEY	LEVEL III KEY
* Parasites: (Termites, carpenter ants, powder post beetles) Observation:			
a. Pinholes with dark stain  area around the holes.  * * * {Severity L}	SF		
b. Holes less than 1/8" diameter, surface sag, and sawdust observed.  * * * {Severity M}	SF	1	
c. Holes greater than 1/8" diameter, surface channels, and crushing of the member.  * * * {Severity H}	SF		2
Defect:			

# \* Deck Missing:

Observation: 2 SF a. Hole in deck. \*\*\*{Severity H}

**LEVEL III** 

**LEVEL II** 

### **16.05 BRIDGE DECKS**

# **COMPONENTS (Continued)**

### **◆ 16.05.03 CONCRETE DECK**

Potential defects which may be observed on concrete decks include cracking, scaling, spalling, corrosion to reinforcement and overloads.

Defect:	UOM	KEY	KEY
* Wear and Abrasion (Top Surface):			
Observation:			
<ul><li>a. Slight or noticeable dull finish to concrete surface.</li><li>***{Severity L}</li></ul>	SF		
<ul> <li>b. Distinctive dull finish to concrete surface.</li> </ul>	SF		
* * * {Severity M}			
<ul> <li>c. Glossy mirror finished to concrete surface.</li> </ul>	SF		
* * * {Severity H}			
Defect:			
* Transverse Cracks (Top Surface):			
Observation:			
a. Hairline cracks less than 1/16" wide.	LF		
* * * {Severity L}			
b. Medium cracks greater than 1/16", less than 1/8" wide with spalling of cracks.	LF	2	
* * * {Severity M}			
c. Wide cracks greater than 1/8" wide with spalling of cracks and reinforcing bars exposed.  ***{Severity H}	LF		3
foeventh 11			

### **COMPONENTS (Continued)**

### ◆ 16.05.03 CONCRETE DECK (Continued)

Defect:	иом	KEY	LEVEL III KEY
* Longitudinal Cracks (Top Surface):			
Observation:			
<ul><li>a. Hairline cracks less than 1/16" wide.</li></ul>	LF		
* * * {Severity L}			
b. Medium cracks greater than 1/16", less than 1/8" wide with spalling of cracks.	LF	2	
* * * {Severity M}			
c. Wide cracks greater than 1/8" wide with spalling of cracks and reinforcing bars exposed.	LF		3
* * * {Severity H}			
• , ,			

### Defect:

### \* Transverse Cracks (Underside):

Observation:

a.	Hairline cracks less than	LF	
	1/16" wide, slight staining of		
	concrete surface.		
* * *	{Severity L}		
b.	Medium cracks greater than	LF	
	1/16" and less than	•	
	1/9" wide Staining of concrete		

1/8" wide. Staining of concrete surface with signs of efflorescence deposits and spalling of crack.

\* \* \* {Severity M}

c. Wide cracks greater than 1/8" wide. Staining of concrete surface with efflorescence deposits, and spalling of crack and reinforcing bars exposed.

\*\*\*{Severity H}

3

2

LF

### **COMPONENTS (Continued)**

### ◆ 16.05.03 CONCRETE DECK (Continued)

UOM	LEVEL II KEY	LEVEL III KEY
LF		
LF	2	
LF		3
	LF LF	LF 2

SF

SF

SF

### Defect:

### \* Scaling:

Observation:

- Loss of surface mortar greater than
   1/4" deep and less than 1/2"
   deep with exposed aggregate.
- \* \* \* {Severity L}
- Loss of surface mortar greater than
   1/2" deep, less than 1" deep. Coarse aggregates are clearly exposed.
- \*\*\*{Severity M}
- c. Loss of coarse aggregate particles, as well as mortar surrounding coarse aggregates; depth of the loss greater than 1" deep, reinforcing bars exposed.
- \* \* \* {Severity H}

### **COMPONENTS (Continued)**

# ◆ 16.05.03 CONCRETE DECK (Continued)

Defect:	UOM	LEVEL II KEY	LEVEL III KEY
* Spalling:			
Observation:			
<ul> <li>Depression less than 1" deep and less than 6" in diameter.</li> </ul>	SF		
* * * (Severity L)			
<ul> <li>b. Depression greater than 1" deep and greater than 6" in diameter.</li> </ul>	SF	2	
* * * {Severity M}			
c. Depression greater than 1" deep and greater than 6" in diameter with corroded reinforcing bars.	SF		3
* * * {Severity H}			

### Defect:

## \* Popout:

Observation:

a.	Conical shape holes less than	SF
	1/2" diameter.	
* * *	'{Severity L}	

b. Conical shape hole greater than SF 1/2", less than 2-1/2" diameter.

\*\*\*{Severity M}

c. Conical shape hole greater than SF 2-1/2" in diameter.

\* \* \* {Severity M}

# **COMPONENTS (Continued)**

### ◆ 16.05.03 CONCRETE DECK (Continued)

V 10.03.03 CONTONETE DECK (Continued)			
Defect:	UOM	LEVEL II KEY	LEVEL III KEY
* Honeycombing:			
Observation:			
<ul> <li>a. Hollow spaces or voids present within concrete, aggregated partially exposed, concrete is sound around damage area.</li> <li>* * * {Severity L}</li> </ul>	SF		
b. Hollow spaces or voids present within concrete with exposed aggregate, concrete is sound around defected area.  ***{Severity M}	SF	2	
<ul> <li>c. Hollow spaces or voids present within concrete with exposed reinforcing bars.</li> <li>***{Severity H}</li> </ul>	SF		3
Defect:			
* Vibration - Per Span:			
Observation:			
a. Slight vibration in deck when vehicles pass.	LF		
* * * {Severity L} b. Noticeable vibration in deck when vehicles pass. * * * {Severity M}	LF		
<pre>* * * {Severity M} c. Excessive vibration in deck when     vehicles pass. * * * {Severity H}</pre>	LF		3

### Defect:

### \* Deck Missing:

Observation:		
a. Hole in deck.	SF	3
* * * {Severity H}		

### **COMPONENTS (Continued)**

### ◆ 16.05.04 STEEL DECK

The common types of steel decks are grid and corrugated steel flooring. Impervious wearing surfaces are often placed over the steel deck to protect the steel from weather and corrosion.

The potential defects which may be observed on steel deck include corrosion, wear, and vehicular damage.

		LEVEL II	LEVEL III
Defect:	UOM	KEY	KEY
* Wear - No Wearing Surface: Observation:			
<ul> <li>a. Minor wear to the serrated bars or steel plate.</li> <li>***{Severity L}</li> </ul>	SF		
<ul> <li>b. Noticeable wear to the serrated bars or plate, surface slippery.</li> <li>*** {Severity M}</li> </ul>	SF		
Defect:			
* Connections:			
Observation:	4		
a. Loose bolts or fasteners.	EA		
* * * {Severity M}			_
<pre>b. Broken or missing bolts, rivets. ***{Severity H}</pre>	EA		4
c. Broken welds.	LF		4
* * * {Severity H}			
Defect:			
* Corrosion - Grid Rails or Corrugated Steel Floori Observation:	ing:		
<ul><li>a. Surface rust no pitting evident.</li><li>***{Severity L}</li></ul>	SF		
b. Corrosion evident pitting and blistering of base material.  * * * {Severity M}	SF		

SF

Corrosion evident with loss to base

section.
\* \* \* {Severity H}

### **COMPONENTS (Continued)**

◆ 16.05.04 STEEL DECK (Continued)

vehicles pass.

vehicles pass.

Noticeable vibration in deck

Excessive vibration in deck when

when vehicles pass.

\* \* \* {Severity L}

\* \* \* {Severity M}

\* \* \* {Severity H}

Defect:	UOM	LEVEL II KEY	LEVEL III KEY
* Damage Grid Rail and/or Transverse Bars: Observation:			
<ul><li>a. Out of alignment or damaged member.</li><li>***{Severity M}</li></ul>	LF		
<ul><li>b. Grid rail cracked.</li><li>***{Severity H}</li></ul>	LF		
<ul><li>c. Broken or missing member.</li><li>***{Severity H}</li></ul>	LF		
Defect:			
* Vibration - Per Span: Observation:			
a. Slight vibration in deck when	LF		

LF

LF

### **COMPONENTS (Continued)**

### **◆ 16.05.05 EXPANSION JOINTS**

The primary function of the expansion joint is to accommodate the expansion and contraction of the bridge deck due to the thermal or other forces. The joint also fills the gap between the deck surface and the backwall. In addition, the deck joint provides a smooth transition from the approach roadway to the bridge deck. The joint opening size depends on the season, the type of expansion joint, the temperature range, and the length of slab whose expansion the joint must accommodate. There are two types of expansion joints, open and closed joints. The open joint, allows water and debris to pass through the joint where as the closed joint prevents water and debris from passing through the joint.

Defect:	UOM	LEVEL II KEY	LEVEL III KEY
* Joint Displacement:			
Observation:			
<ul> <li>a. Vertical displacement less than 1/4".</li> </ul>	LF		
* * * {Severity L}			
b. Vertical displacement greater than 1/4", less than 1/2".	LF		
* * * {Severity M}			
c. Vertical displacement greater than 1/2".	LF		5
* * * {Severity H}			

#### Defect:

#### \* Joint Horizontal Clearance:

The proper joint opening size depends on the season, the type of expansion joint, the temperature range, and the length of the slab whose expansion the joint must accommodate.

#### Observation:

a. Joint opening less than 3".	LF	
* * * {Severity M}		
b. Joint completely closed	LF	5
* * * {Severity H}		
c. Joint opening greater than 4".	LF ·	5
* * * {Severity H}		

### **COMPONENTS (Continued)**

### **◆** 16.05.05 EXPANSION JOINTS (Continued)

Defect:	иом	LEVEL II KEY	LEVEL III KEY
* Finger Plate:			
Observation:			
a. Loose finger plate.	LF		5
* * * {Severity M}	·		_
b. Jammed fingers.	LF		5
* * * {Severity M}			
c. Fingers do not lap.	LF		5
* * * {Severity H}			
<ul> <li>d. Broken or cracked joint finger.</li> </ul>	EA		5
* * * {Severity H}			
<ul> <li>e. Broken fasteners or welds.</li> </ul>	EA		5
* * * {Severity H}			· ·
Defect:			

### 

### \* Corrosion, Expansion or Armor Plates:

Obser	vat	ion:	

a.	Surface rust no pitting evident.	SF
* * *	{Severity L}	

Corrosion evident pitting and SF blistering of base material.

\* \* \* {Severity H}

\* \* \* {Severity M} Corrosion evident with loss to base SF 5 section.

COMPONENTS (Continued)			
◆ 16.05.05 EXPANSION JOINTS (Continued)			
Defect:	UOM	LEVEL II KEY	LEVEL III KEY
* Joint Seal Damage:		•	
Observation:  a. Joint sealant is in good condition throughout the section, allowing little water and no incompressible material to infiltrate through the joint.	LF		
***{Severity L} b. Joint condition is fair throughout the section. Water can infiltrate the joint fairly easily and some incompressible materials can infiltrate the joint.	LF		
***{Severity M} c. Joint sealant is in poor condition throughout the section. Water and incompressible material can freely infiltrate the joint.  ***{Severity H}	LF .		5
Defect:			
* Sliding Plate:			
Observation:  a. Loose sliding plates or anchor.  * * * {Severity M}	LF		
b. Jammed slide plate.  ***{Severity M}	LF		
<ul> <li>c. Bent or cracked sliding plate.</li> </ul>	LF		5
<pre>***{Severity H } d. Broken fasteners or welds. ***{Severity H }</pre>	EA		5
Defect:			
* Indiscriminate Overlay:			
Observation:  a. New pavement or wearing surface  over existing deck joint.	SF		

\*\*\*{Severity H}

#### REFERENCES

- 1. U.S. Department of Transportation, Federal Highway Administration, Bureau of Public roads (DOT) Bridge Inspector's Training Manual, 1990 Edition
- 2. Bridge Inspection and Rehabilitation, Parsons Brinckerhoff Edited by L.G. Silano, PE, 1993
- 3. Inspection of Bridges and Trestles NAVFAC MO-126, October 1991
- 4. ACI Manual of Concrete Practice 1989, Part 1 Materials and General Properties of Concrete
- 5. AASHTO Manual for Maintenance Inspection of Bridges, American Association of State Highway and Transportation Officials
- 6. Micro Bridger (Version 1.0), U.S. Army Construction Engineering Research Laboratory
- 7. AASHTO Guide for Design of Pavement Structures, 1986
- 8. TM 5-623, Pavement Maintenance Management, November 1982
- 9. Principals of Pavement Design, E. J. Yoder, John Wiley & Sons, Inc.
- Micro PAVER, User's Guide, Version 3.0, U.S. Army Corps of Engineers, Construction Engineering Research Laboratory, January 1992

	•
LEVEL II KEY	GUIDE SHEET CONTROL NUMBER
1	GS-II 16.05.01-1
2	GS-II 16.05.02-2
LEVEL III KEY	GUIDE SHEET CONTROL NUMBER
LEVEL III KET	GOIDE STIEL CONTROL NOWIDER
LEVEL III KEY	GOIDE SHEET CONTROL NOWIDEN
1	GS-III 16.05.01-1
1	GS-III 16.05.01-1
1 2 3	GS-III 16.05.01-1 GS-III 16.05.02-2 GS-III 16.05.03-3
1 2	GS-III 16.05.01-1 GS-III 16.05.02-2

### **LEVEL II GUIDE SHEET - KEY NO. 1**

COMPONENT: TIMBER DECK
CONTROL NUMBER: GS-II 16.05.02-1

### **Application**

This applies to the investigation of possible interior and exterior deterioration of timber deck members due to decay (rot), or parasites.

The results of the Level II inspection can be used to trigger a level III inspection or necessary repair.

### **Special Safety Requirements**

No special Safety Requirements, beyond the requirements listed in the Safety Section, are to be observed in the performance of the Level II.

### **Inspection Actions**

- 1. Clean affected area.
- 2. Measure affected area.
- 3. Tap with hammer to determine extend of hollow or sound material.
- 4. Probe with ice pick.

### Recommended inspection Frequency

As triggered by Level I or Level II defect/observation.

### References

- U.S. Department of transportation, Federal Highway Administration, Bureau of Public Roads (DOT) Bridge Inspector's Training Manual, 1990 Edition
- 2. Bridge inspection and Rehabilitation, Parsons Brinckerhoff Edited by L.G. Silano, PE, 1993
- 3. Inspection of Bridges and Trestles NAVFAC MO-126, October 1991
- 4. AASHTO Manual for Maintenance Inspection of Bridges, American Association of State Highway and Transportation Officials
- 5. Micro Bridger (Version 1.0), U.S. Army Construction Engineering Research Laboratory

### LEVEL II GUIDE SHEET - KEY NO. 2

COMPONENT:

CONCRETE DECK

CONTROL NUMBER: GS-II 16.05.03-2

#### **Application**

This applies to the investigation of concrete bridge deck deterioration due to spalling from delamination.

The results of the Level II inspection can be used to trigger a level III inspection or necessary

### **Special Safety Requirements**

No special Safety Requirements, beyond the requirements listed in the Safety Section, are to be observed in the performance of the level II.

### **Inspection Actions**

- 1. Clean loose concrete from area to be inspected.
- 2. Measure the affected area.
- Tap the affected area with a hammer to determine extent of unsound or hollow 3. concrete.

### Recommended Inspection Frequency

As triggered by Level I or Level II defect/observation

### References

- 1. U.S. Department of transportation, Federal Highway Administration, Bureau of Public roads (DOT) Bridge Inspector's Training Manual, 1990 Edition
- 2. Bridge inspection and Rehabilitation, Parsons Brinckerhoff Edited by L.G. Silano, PE, 1993
- 3. Inspection of Bridges and Trestles NAVFAC MO-126, October 1991
- 4. ACI Manual of Concrete Practice 1989, Part 1 Materials and General Properties of Concrete
- 5. AASHTO Manual for Maintenance Inspection of Bridges, American Association of State Highway and Transportation Officials
- 6. Micro Bridger (Version 1.0), U.S. Army Construction Engineering Research Laboratory

#### LEVEL III GUIDE SHEET - KEY NO. 1

**COMPONENT:** 

ASPHALTIC WEARING SURFACE

CONTROL NUMBER: GS-III 16.05.01-1

#### **Application**

This guide applies to investigation and testing of asphaltic wearing surface, to determine their structural capacity and to perform maintenance.

### **Special Safety Requirements**

Passing traffic is a hazard. Level III inspection and testing must be performed with the prior approval of the Facility Manager who will notify the authorities to provide safety measures and safe access.

### **Inspection Actions**

Results of Level I inspection yield a measure of surface integrity of the pavement surfaces. Although Level I inspection methodology is very useful for maintaining the pavement systems of the base, its analysis however, cannot determine structural capacity of the pavement. When the pavement condition dictates that its rehabilitation may be required, then a more extensive Level III Inspection is essential. Level III requires the use of a more advanced method of testing includes techniques to measure pavement deflection, and advanced testing of one or more pavement components to determine component properties and strength. This equipment includes:

- Benkleman Beam
- Dynaflect
- Falling Weight Deflectometer
- Skid Resistance testing

These techniques can be used to detect voids under the pavement by the use of Ground Penetrating Radar equipment. Advanced techniques include sample coring through concrete or asphalt pavements to determine thickness, strength, and composition.

### LEVEL III GUIDE SHEET - KEY NO. 1 (Continued)

**COMPONENT:** 

ASPHALTIC WEARING SURFACE

CONTROL NUMBER: GS-III 16.05.01-1

### **Special Tools and Equipment**

Standard testing equipment required to perform the advanced method testing and inspection

### Recommended Inspection Frequency

As triggered by Level I or Level II defect/observation

### References

- AASHTO Guide for Design of Pavement Structures, 1986
- 2. TM 5-623, Pavement Maintenance Management, November 1982
- 3. Principals of Pavement Design, E. J. Yoder, John Wiley & Sons, Inc.
- 4. Micro PAVER, User's Guide, Version 3.0, U.S. Army Corps of Engineers, Construction Engineering Research Laboratory, January 1992
- 5. ASTM D 5340 93, Standard Test Method for Airport Pavement Condition Index Surveys
- 6. TM 5-826-6/ AFR 93-5, Procedures for US Army and US Air Force Airfield Condition Surveys, July 1989

### LEVEL III GUIDE SHEET - KEY NO. 2

COMPONENT:

TIMBER DECK

CONTROL NUMBER: GS-III 16.05.02-2

#### Application

This guide has been prepared to identify the purpose of a Level III inspection and its more sophisticated test and inspection methods which may be appropriate to determine the cause and/or extent of defects recorded in Level I or Level II defect observations on a timber deck.

Whereas the purpose of the Level I inspection was to record the observable defects on the timber deck, this Level III inspection is performed to provide a thorough systematic evaluation of the observed defect and to make an assessment of it's effects, if left unchecked, on the safety, durability and stability of the structural deck and it's appurtenant works.

The Level III inspection should be performed when prompted by the results of a Level I or II inspection. The inspection should be performed by an engineer or multidisciplined team of engineers experienced in the design and construction of bridges.

The results of the Level III inspection will be used to develop maintenance or remedial measure work strategy that will correct the existing deficiency conditions or to require continued monitoring of existing deficiency conditions on the bridge deck.

In general, appropriate advanced inspection methods will be identified, recommended, and performed by or under the supervision of the inspection engineer personnel as part of the Level III test and inspection method. Advanced inspection methods will be assigned only after the assessment of defect conditions observed during a Level I or II inspection.

#### **Special Safety Requirements**

Passing traffic is a hazard. Level III inspection and testing must be performed with the prior approval of the Facility Manager who will notify the authorities to provide safety measures and safe access.

### LEVEL III GUIDE SHEET - KEY NO. 2 (Continued)

COMPONENT: TIMBER DECK CONTROL NUMBER:

GS-III 16.05.02-2

### **Inspection Action**

1. Prior to making a field inspection of the observed defect, review all past records concerning the deck and the defective component if available. These records may include pre-construction investigation records, design criteria and analysis records, available construction records, previous periodic maintenance inspection records, water level records, and photographs taken during initial construction and subsequent inspections.

- 2. Perform inspection of the pertinent components where observed defects that triggered a Level III inspection are listed.
- 3. Make an assessment of the importance of individual defects observed for a given component at the bridge site. Indicate priorities for any required maintenance, or remedial measure work.
- 4. Identify whether particular observed defects need additional or continued observation.
- 5. Assess the stability and safety of the bridge deck.
- 6. Prepare final cost estimate for advanced inspection methods required to determine the cause and extent of the observed defect.
- 7. Prepare cost estimate for required maintenance or remedial repair measures, as applicable.

Level III advanced inspection methods may be required for specific Level I and Level II defect conditions observed at a bridge site. Level III advanced test or inspection methods and associated observed defects for bridge deck include, but are not limited to the following:

#### LEVEL III GUIDE SHEET - KEY NO. 2 (Continued)

COMPONENT:

TIMBER DECK

CONTROL NUMBER: GS-III 16.05.02-2

#### Advanced Test or Inspection Method **Applicable Observed Defects**

1. Increment borer

interior and exterior deterioration of timber due to

decay or parasites

2. Ultrasonic

interior deterioration

3. Moisture content

deterioration due to decay or parasites

4. Survey measurements

deck movement

### **Special Instructions**

Review as-built and design drawings of structure.

### **Special Tools & Equipment Requirements**

Increment borer Surveying equipment Navigable boat with related safety equipment Moisture meter

### **Recommended Inspection Frequency**

As triggered by Level I and II defect/observations or every 3 years.

#### References

- 1. U.S. Department of transportation, Federal Highway Administration, Bureau of Public roads (DOT) Bridge Inspector's Training Manual, 1990 Edition
- Bridge inspection and Rehabilitation, Parsons Brinckerhoff Edited by L.G. Silano, PE, 2. 1993
- Inspection of Bridges and Trestles NAVFAC MO-126, October 1991 3.
- AASHTO Manual for Maintenance Inspection of Bridges, American Association of State Highway and Transportation Officials
- Micro Bridger (Version 1.0), U.S. Army Construction Engineering Research Laboratory

### **LEVEL III GUIDE SHEET - KEY NO. 3**

COMPONENT: CONTROL NUMBER: CONCRETE DECK

GS-III 16.05.03-3

### **Application**

This guide has been prepared to identify the purpose of a Level III inspection and its more sophisticated test and inspection methods which may be appropriate to determine the cause and/or extent of defects recorded in Level I or Level II defect observations on a concrete deck.

Whereas the purpose of the Level I inspection was to record the observable defects on the deck, this Level III inspection is performed to provide a thorough systematic evaluation of the observed defect and to make an assessment of it's effects, if left unchecked, on the safety, durability and stability of the structural deck and it's appurtenant works.

The Level III inspection should be performed when prompted by the results of a Level I or II inspection. The inspection should be performed by an engineer or multidisciplined team of engineers experienced in the design and construction of bridges.

The results of the Level III inspection will be used to develop maintenance or remedial measure work strategy that will correct the existing deficiency conditions or to require continued monitoring of existing deficiency conditions on the bridge deck.

In general, appropriate advanced inspection methods will be identified, recommended, and performed by or under the supervision of the inspection engineer personnel as part of the Level III test and inspection method. Advanced inspection methods will be assigned only after the assessment of defect conditions observed during a Level I or II inspection.

#### **Special Safety Requirements**

Passing traffic is a hazard. Level III inspection and testing must be performed with the prior approval of the Facility Manager who will notify the authorities to provide safety measures and safe access.

### LEVEL III GUIDE SHEET - KEY NO. 3 (Continued)

COMPONENT: CONTROL NUMBER: CONCRETE DECK

GS-III 16.05.03-3

#### Inspection Action

- 1. Prior to making a field inspection of the observed defect, review all past records concerning the concrete deck and the defective component if available. These records may include pre-construction investigation records, design criteria and analysis records, available construction records, previous periodic maintenance inspection records, water level records, and photographs taken during initial construction and subsequent inspections.
- 2. Perform inspection of the pertinent components where observed defects that triggered a Level III inspection are listed.
- Make an assessment of the importance of individual defects observed for a given component at the bridge site. Indicate priorities for any required maintenance, or remedial measure work.
- 4. Identify whether particular observed defects need additional or continued observation.
- 5. Assess the stability and safety of the bridge deck.
- 6. Prepare final cost estimate for advanced inspection methods required to determine the cause and extent of the observed defect.
- 7. Prepare cost estimate for required maintenance or remedial repair measures, as applicable.

Level III advanced inspection methods may be required for specific Level I and Level II defect conditions observed at a bridge site. Level III advanced test or inspection methods and associated observed defects for bridge deck include, but are not limited to the following:

### LEVEL III GUIDE SHEET - KEY NO. 3 (Continued)

COMPONENT:

CONCRETE DECK

CONTROL NUMBER:

GS-III 16.05.03-3

### 

1. Infrared Thermography and ground probing radar

concrete spalling and delamination

2. Concrete coring

concrete deterioration, cracking, spalling

 Laboratory test on concrete (core, strength tests, abrasion, absorption,sulfate soundness, unit weight concrete deterioration and strength

4. Ultrasonic test

internal cracks and spalling, delamination

5. Half-cell test

corrosion to reinforcement steel

6. Survey measurements

deck movement

### **Special Instructions**

Review as-built and design drawings of structure.

### **Special Tools & Equipment Requirements**

Industry required testing equipment needed to perform the advanced investigation method chosen.

Navigable boat with related safety equipment.

Surveying equipment

### **Recommended Inspection Frequency**

As triggered by Level I and II defect/observations or every 3 years.

# LEVEL III GUIDE SHEET - KEY NO. 3 (Continued)

COMPONENT:

**CONCRETE DECK** 

CONTROL NUMBER:

GS-III 16.05.03-3

# References

1. U.S. Department of transportation, Federal Highway Administration, Bureau of Public roads (DOT) Bridge Inspector's Training Manual, 1990 Edition

- 2. Bridge inspection and Rehabilitation, Parsons Brinckerhoff Edited by L.G. Silano, PE, 1993
- 3. Inspection of Bridges and Trestles NAVFAC MO-126, October 1991
- 4. ACI Manual of Concrete Practice 1989, Part 1 Materials and General Properties of Concrete
- 5. AASHTO Manual for Maintenance Inspection of Bridges, American Association of State Highway and Transportation Officials
- 6. Micro Bridger (Version 1.0), U.S. Army Construction Engineering Research Laboratory

#### LEVEL III GUIDE SHEET - KEY NO. 4

COMPONENT: CONTROL NUMBER: STEEL DECK

GS-III 16.05.04-4

#### **Application**

This guide has been prepared to identify the purpose of a Level III inspection and its more sophisticated test and inspection methods which may be appropriate to determine the cause and/or extent of defects recorded in Level I or Level II defect observations on a steel deck.

Whereas the purpose of the Level I inspection was to record the observable defects at the expansion joint, this Level III inspection is performed to provide a thorough systematic evaluation of the observed defect and to make an assessment of it's effects, if left unchecked, on the safety, durability and stability of the structural deck and it's appurtenant works.

The Level III inspection should be performed when prompted by the results of a Level I or II inspection. The inspection should be performed by an engineer or multidisciplined team of engineers experienced in the design and construction of bridges.

The results of the Level III inspection will be used to develop maintenance or remedial measure work strategy that will correct the existing deficiency conditions or to require continued monitoring of existing deficiency conditions on the bridge deck.

In general, appropriate advanced inspection methods will be identified, recommended, and performed by or under the supervision of the inspection engineer personnel as part of the Level III test and inspection method. Advanced inspection methods will be assigned only after the assessment of defect conditions observed during a Level I or II inspection.

### **Special Safety Requirements**

Passing traffic is a hazard. Level III inspection and testing must be performed with the prior approval of the Facility Manager who will notify the authorities to provide safety measures and safe access.

#### LEVEL III GUIDE SHEET - KEY NO. 4 (Continued)

COMPONENT:

STEEL DECK

CONTROL NUMBER: GS-I

GS-III 16.05.04-4

#### **Inspection Action**

 Prior to making a field inspection of the observed defect, review all past records concerning the steel deck and the defective component if available. These records may include pre-construction investigation records, design criteria and analysis records, available construction records, previous periodic maintenance inspection records, water level records, and photographs taken during initial construction and subsequent inspections.

- 2. Perform inspection of the pertinent components where observed defects that triggered a Level III inspection are listed.
- 3. Make an assessment of the importance of individual defects observed for a given component at the bridge site. Indicate priorities for any required maintenance, or remedial measure work.
- 4. Identify whether particular observed defects need additional or continued observation.
- 5. Assess the stability and safety of the bridge deck.
- 6. Prepare final cost estimate for advanced inspection methods required to determine the cause and extent of the observed defect.
- 7. Prepare cost estimate for required maintenance or remedial repair measures, as applicable.

Level III advanced inspection methods may be required for specific Level I and Level II defect conditions observed at a bridge site. Level III advanced test or inspection methods and associated observed defects for bridge deck include, but are not limited to the following:

#### LEVEL III GUIDE SHEET - KEY NO. 4 (Continued)

COMPONENT:

STEEL DECKS

CONTROL NUMBER:

GS-III 16.05.04-4

#### 

1. Grinding and or sandblasting, using caliper to measure section loss

corrosion of steel and section loss

2. Magnetic particle

cracks in steel or welds

3. Dye-Penetrant

cracks in steel or welds

4. Ultrasonic

cracks and voids in steel

5. Survey measurements

deck movement

### **Special Instructions**

Review as-built and design drawings of structure.

#### **Special Tools & Equipment Requirements**

Grinder or sand blasting equipment
Surveying equipment
Navigable boat with related safety equipment
Industry required testing equipment needed to perform the advanced investigation
method chosen

#### **Recommended Inspection Frequency**

As triggered by Level I and II defect/observations or every 3 years.

#### References

- U.S. Department of transportation, Federal Highway Administration, Bureau of Public roads (DOT) Bridge Inspector's Training Manual, 1990 Edition
- 2. Bridge inspection and Rehabilitation, Parsons Brinckerhoff Edited by L.G. Silano, PE, 1993
- 3. Inspection of Bridges and Trestles NAVFAC MO-126, October 1991
- 4. AASHTO Manual for Maintenance Inspection of Bridges, American Association of State Highway and Transportation Officials
- 5. Micro Bridger (Version 1.0), U.S. Army Construction Engineering Research Laboratory

# **LEVEL III GUIDE SHEET - KEY NO. 5**

COMPONENT:

**EXPANSION JOINTS** 

CONTROL NUMBER: (

GS-III 16.05.05-5

#### **Application**

This guide has been prepared to identify the purpose of a Level III inspection and its more sophisticated test and inspection methods which may be appropriate to determine the cause and/or extent of defects recorded in Level I or Level II defect observations at the expansion joint.

Whereas the purpose of the Level I inspection was to record the observable defects at the expansion joint, this Level III inspection is performed to provide a thorough systematic evaluation of the observed defect and to make an assessment of it's effects, if left unchecked, on the safety, durability and stability of the expansion joints and it's appurtenant works.

The Level III inspection should be performed when prompted by the results of a Level I or II inspection. The inspection should be performed by an engineer or multidisciplined team of engineers experienced in the design and construction of bridges.

The results of the Level III inspection will be used to develop maintenance or remedial measure work strategy that will correct the existing deficiency conditions or to require continued monitoring of existing deficiency conditions on the expansion joint.

In general, appropriate advanced inspection methods will be identified, recommended, and performed by or under the supervision of the inspection engineer personnel as part of the Level III test and inspection method. Advanced inspection methods will be assigned only after the assessment of defect conditions observed during a Level I or II inspection.

# **Special Safety Requirements**

Passing traffic is a hazard. Level III inspection and testing must be performed with the prior approval of the Facility Manager who will notify the authorities to provide safety measures and safe access.

#### **LEVEL III GUIDE SHEET - KEY NO. 5 (Continued)**

COMPONENT: CONTROL NUMBER: **EXPANSION JOINTS** 

GS-III 16.05.05-5

#### Inspection Action

 Prior to making a field inspection of the observed defect, review all past records concerning the expansion joints and the defective component if available. These records may include pre-construction investigation records, design criteria and analysis records, available construction records, previous periodic maintenance inspection records, water level records and photographs taken during initial construction and subsequent inspections.

- 2. Perform inspection of the pertinent components where observed defects that triggered a Level III inspection are listed.
- 3. Make an assessment of the importance of individual defects observed for a given component at the bridge site. Indicate priorities for any required maintenance, or remedial measure work.
- 4. Identify whether particular observed defects need additional or continued observation.
- 5. Assess the stability and safety of the expansion joint.
- 6. Prepare final cost estimate for advanced inspection methods required to determine the cause and extent of the observed defect.
- 7. Prepare cost estimate for required maintenance or remedial repair measures, as applicable.

Level III advanced inspection methods may be required for specific Level I and Level II defect conditions observed at a bridge site. Level III advanced test or inspection methods and associated observed defects for expansion joint include, but are not limited to the following:

# LEVEL III GUIDE SHEET - KEY NO. 5 (Continued)

COMPONENT:

**EXPANSION JOINTS** 

CONTROL NUMBER:

GS-III 16.05.05-5

#### 

1. Grinding or sandblasting, using caliper to measure section loss

corrosion of steel and section loss

2. Magnetic particle

cracks in steel or welds

3. Dye-Penetrant

cracks in steel or welds

4. Ultrasonic test

cracks and voids in steel

5. Soil boring

Settlement of substructure

6. Survey measurements

deck movement

# **Special Instructions**

Review as-built and design drawings of structure.

### Special Tools & Equipment Requirements

Navigable boat with related safety equipment
Surveying equipment
Grinder or sand blaster equipment
Industry required testing equipment needed to perform the advanced investigation

Industry required testing equipment needed to perform the advanced investigation method chosen

# **Recommended Inspection Frequency**

As triggered by Level I and II defect/observations or every 3 years.

#### References

- 1. U.S. Department of transportation, Federal Highway Administration, Bureau of Public roads (DOT) Bridge Inspector's Training Manual, 1990 Edition
- 2. Bridge inspection and Rehabilitation, Parsons Brinckerhoff Edited by L.G. Silano, PE, 1993
- 3. Inspection of Bridges and Trestles NAVFAC MO-126, October 1991
- AASHTO Manual for Maintenance Inspection of Bridges, American Association of State Highway and Transportation Officials
- 5. Micro Bridger (Version 1.0), U.S. Army Construction Engineering Research Laboratory

#### **DESCRIPTION**

The bridge railing or parapets is a fence-like construction built at the outer most edge of the roadway or sidewalk portion of a bridge. The primary function of the bridge railing or parapet is safety, to keep errant vehicles from driving off the edge of the bridge and to guard and guide the movement of pedestrian traffic. Bridge railing must also smoothly redirect the vehicle in such a manner that the vehicle does not overturn and the railing does not fail.

On older steel or timber bridge railing consisted of timber planks nailed together in a picket fence like arrangement. More recently, railings are made of steel angles, welded steel tubing or "W" shape rail supported by a wide flange post replacing the timber railing. Solid concrete parapet with steel or aluminum railing attached to the top are also commonly used today.

# SPECIAL TOOL AND EQUIPMENT REQUIREMENTS

The following list of special tools and equipment, beyond the requirements listed in the Standard Tools Section, may be required to perform the inspection of this subsystem:

- 1. Boat
- 2. Related safety equipment

# SPECIAL SAFETY REQUIREMENTS

Level I and Level II inspection is performed by walking the bridge deck, and observing from ground level, thus passing traffic is a hazard. The inspection must be performed with the prior approval of the Facility Manager, who will notify the necessary authorities to provide traffic safety measures and access. Inspector will be required to wear orange safety vest.

Depending on the bridge type a boat may be required to observe certain components. Inspectors are required to take all necessary safety measures, and refer to the Master Safety Plan for guidance and compliance.

# COMPONENT LIST

- ◆ 16.06.01 TIMBER RAILING AND POST
- ◆ 16.06.02 CONCRETE PARAPETS
- ♦ 16.06.03 STEEL RAILING AND POST

# RELATED SUBSYSTEMS

Due to the related nature of the elements requiring inspection, the following should be reviewed for concurrent inspection activities.

19.01	ROADWAYS
20.01	RAILROAD

29.00 SITE ELECTRICAL (all subsystems)

# STANDARD INSPECTION PROCEDURE

The Level I inspection shall be carried out for each component in the order listed. The inspector will identify any physical defects existing in each component; will note the level of severity of each defect type; and measure the quantity of each defect.

# **COMPONENTS**

# **◆ 16.06.01 TIMBER RAILING AND POST**

The potential defects which may be observed in timber railing include decay, parasite damage and deterioration from weather and vehicle damage.

Defect:	UOM	LEVEL II KEY	LEVEL III KEY
* Weathering:			
Observation:			
<ul> <li>a. Surface of wood is rough</li> <li>and corrugated and member</li> <li>may be warped.</li> </ul>	LF		
***{Severity L}			
<ul> <li>Surface of wood is rough and corrugated with cracks partially through the wood member, may have minor section loss. Member may be warped</li> </ul>	LF d.		
* * * {Severity M} c. Large cracks extend deeply or completely through the wood.	LF		
<pre>***{Severity H} d. Wood is crumbly and seriously</pre>	LF		1

#### Defect:

# \* Decay (Rot/Fungus Decay):

Decay from rot/fungus is most likely to occur at connections, splices, support points or around bolt holes. This may be due either to the tendency of such areas to collect and retain moisture, or to bolt holes or cuts being made in the surface after the preservative treatment has been applied.

#### Observation:

Ob	servation.	
a.	Moist and stained or discolored area,	LF
	signs of fungi, surface is solid.	
* *	*{Severity L}	
b.	Surface spongy, member may shown	LF
	signs of crushing	
* *	*{Severity M}	
c.	Brown and white - discolored area,	LF
	member may show section loss and	

crushing.
\* \* \* {Severity H}

# **COMPONENTS (Continued)**

# ◆ 16.06.01 TIMBER RAILING AND POST (Continued)

Defect:	UOM	LEVEL II	KEY
* Parasites:			
(Termites, carpenter ants, powder post beetles	s)		
Observation:			
a. Pinholes with dark stain	LF		
area around the holes.			
***{Severity L}	, ,-		
b. Holes less than 1/8" diameter,	LF	•	
surface sag, and sawdust observed.			
* * * {Severity M}			•
c. Holes greater than 1/8" diameter,	LF		1
surface channels, and crushing of			
the member.			
* * * {Severity H}			
Defect:			
* Post Connections:			
Observation:  a. Loose fasteners.	EA		
a. Loose fasteners.  ***{Severity L}	EA		
b. Broken, split or damaged.	EA		
* * * {Severity H}	_, .		
c. Missing fasteners or anchorage.	EA		
* * * {Severity H}		•	
Defect:			
* Corrosion at Post Anchors:			
Observation:			
<ul> <li>a. Surface rust no pitting evident.</li> </ul>	EA		
* * * {Severity L}			
<ul> <li>b. Corrosion evident pitting and</li> </ul>	EA		
blistering of base material.			
*** { severity M}	ΕΛ		
<ul> <li>c. Corrosion evident with loss to base section.</li> </ul>	EA		
* * * {Severity H}			
(3373, 11)	,		

# **COMPONENTS (Continued)**

◆ 16.06.01 TIMBER RAILING AND POST (Continued)

Defect:	UOM	LEVEL II KEY	KEY
* Vehicular Damage:			
Observation:			
<ul><li>a. Railing out of alignment.</li><li>***{Severity M}</li></ul>	LF		
<ul><li>b. Post out of alignment.</li><li>* * * {Severity M}</li></ul>	LF		
<ul><li>c. Railing shattered or damaged.</li><li>***{Severity H}</li></ul>	LF		
<ul><li>d. Post shattered or damaged member.</li><li>* * * {Severity H}</li></ul>	LF		
Defect:			
* Vertical Joint Displacement:			
Due to substructure or bearing settlement.  Observation:			
<ul><li>a. Displacement less than 1/4".</li><li>* * * {Severity L}</li></ul>	EA		
<ul><li>b. Displacement greater than 1/4"</li><li>less than 1/2".</li><li>***{Severity M}</li></ul>	EA		
c. Displacement greater than 1/2".  * * * {Severity H}	EA		
Defect:			
* Horizontal Joint Misalignment:			
Due to substructure or bearing settlement.			
Observation:  a. Displacement less than 1/4".	EA		
* * * {Severity L} b. Displacement greater than 1/4", less than 1/2".	EA		
<pre>***{Severity M} c. Displacement greater than 1/2". ***{Severity H}</pre>	EA		

LEVEL III

LEVEL II

# **16.06 BRIDGE RAILING AND PARAPETS**

# **COMPONENTS (Continued)**

# **◆ 16.06.02 CONCRETE PARAPETS**

Potential defects which may be observed include collision damage and deterioration to the concrete surfaces.

Defect:	UOM	KEY	KEY
* Horizontal Cracks: Observation:			
<ul> <li>a. Hairline cracks less than 1/16"</li> <li>wide, slight staining of concrete.</li> <li>* * * {Severity L}</li> </ul>	LF		
<ul> <li>b. Medium crack greater than 1/16, less than 1/8" wide with spalling along each side of crack. Staining of concrete surface.</li> <li>***{Severity M}</li> </ul>	LF ·		
c. Wide cracks greater than 1/8" wide, with spalling each side of cracks with reinforcing bars exposed ***{Severity H}	LF		
Defect:			
* Vertical Cracks: Observation:			
a. Hairline cracks less than 1/16"  wide, slight staining of concrete.  * * * {Severity L}	LF		
b. Medium cracks greater than 1/16", less than 1/8" wide with spalling of cracks and staining of concrete surface.  ***{Severity M}	LF		
c. Wide cracks greater than 1/8" wide with spalling of cracks and reinforcing bars exposed.  ***{Severity H}	LF		

COMPONENTO (Occasioned)			
COMPONENTS (Continued)			
◆ 16.06.02 CONCRETE PARAPETS (Continued)		151/51 11	
Defect:	UOM	LEVEL II	KEY
* Scaling:			
Observation:	SF		
<ul> <li>a. Loss of surface mortar greater than</li> <li>1/4" deep and less than 1/2"</li> </ul>	5F		
deep with exposed aggregate.			
* * * {Severity L}	05		
b. Loss of surface mortar greater than	SF		
1/2" deep, less than 1" deep. Coarse aggregates are clearly exposed.			
* * * {Severity M}			
<ul> <li>c. Loss of coarse aggregate particles,</li> </ul>	SF		
as well as mortar surrounding coarse aggregates; depth of the loss			
greater than 1" deep, reinforcing bars			
exposed.			
* * * {Severity H}			
Defect:			
* On alling			
* Spalling: Observation:			
<ul> <li>a. Depression less than 1" deep and less</li> </ul>	SF		
than 6" in diameter.			
* * * (Severity L) b. Depression greater than 1" deep and	SF		
greater than 6" in diameter.			
* * * {Severity M}	SF		
<ul> <li>Depression greater than 1" deep and greater than 6" in diameter with</li> </ul>	SF		
corroded re-bars.			
* * * {Severity H}			
Defect:			
* Popout:			
Observation:	C.F.		
<ul> <li>a. Conical shape holes less than</li> <li>1/2" diameter.</li> </ul>	SF		
* * * {Severity L}			
<ul> <li>b. Conical shape Hole greater than</li> </ul>	SF		
1/2" less than 2-1/2" diameter.			
***{Severity M} c. Conical shape hole greater than	SF		
2-1/2" in diameter.			
* * * {Severity M}			

	•		
COMPONENTS (Continued)			
◆ 16.06.02 CONCRETE PARAPETS (Continued)			
Defect:	UOM	LEVEL II KEY	KEY
Defect:			
* Vehicular Damage:			
Observation:  a. Member out of alignment.  * * * {Severity H}	LF		
b. Shattered or major damage to member.  * * * {Severity H}	LF		
Defect:			
* Vertical Joint Displacement:  Due to substructure or bearing settlement.  Observation:			
<ul><li>a. Displacement less than 1/4".</li><li>***{Severity L}</li></ul>	EA		
b. Displacement greater than 1/4", less than 1/2".  ***{Severity M}	EA		
c. Displacement greater than 1/2". * * * {Severity H}	EA		
Defect:			
* Horizontal Joint Misalignment:  Due to substructure or bearing settlement.  Observation:			
a. Displacement less than 1/4".  ***{Severity L}	EA		
b. Displacement greater than 1/4", less than 1/2".  * * * {Severity M}	EA		
c. Displacement greater than 1/2".  * * * {Severity H}	EA		

# **COMPONENTS (Continued)**

# ♦ 16.06.03 STEEL RAILING AND POST

Potential defects which may be observed includes corrosion, collision and out of plane distortion.

Defect:	UOM	LEVEL II KEY	LEVEL III KEY
* Corrosion Railing or Post: Observation:			
<ul> <li>a. Surface rust no pitting evident.</li> </ul>	LF		
***{Severity L} b. Corrosion evident pitting and	LF		
blistering of base material.	<b>L</b> 1		
* * * {Severity M}	LF		
<ul> <li>c. Corrosion evident with loss to base section.</li> </ul>	LF		
* * * {Severity H}			
Defect:			
* Post Connections: Observation:			
a. Loose bolts or fasteners.	EA		
* * * {Severity M}	EA		
<pre>b. Broken or missing bolts. ***{Severity H}</pre>	EA		
Defect:			
* Rail Connections:			
Observation:  a. Loose bolts or fasteners.	EA		
<ul><li>a. Loose bolts or fasteners.</li><li>***{Severity M}</li></ul>			
<ul> <li>b. Broken or missing bolts.</li> </ul>	EA		
* * * {Severity H}			
Defect:			
* Corrosion at Post Anchors:			
Observation:	EA		
<ul><li>a. Surface rust no pitting evident.</li><li>***{Severity L}</li></ul>	LA		
<ul> <li>b. Corrosion evident pitting and</li> </ul>	EA		
blistering of base material.			
<pre>***{Severity M} c. Corrosion evident with loss to base</pre>	EA		
section.			
* * * {Severity H}			

# **COMPONENTS (Continued)**

# ◆ 16.06.03 STEEL RAILING AND POST (Continued)

Defect:	UOM	LEVEL II KEY	LEVEL III KEY
* Vehicular Damage:			
Observation:			
<ul><li>a. Railing out of alignment.</li><li>***{Severity M}</li></ul>	LF		
<pre>b. Post out of alignment. ***{Severity M}</pre>	LF		
<ul><li>c. Post broken or missing.</li><li>***{Severity H}</li></ul>	LF		
<pre>d. Railing broken or missing. ***{Severity H}</pre>	LF		
Defect:			
<ul> <li>Vertical Joint Displacement:</li> <li>Due to substructure or bearing settlement</li> </ul>			
Observation:  a. Displacement less than 1/4".  * * * {Severity L}	EA		
b. Displacement greater than 1/4" less than 1/2".  * * * {Severity M}	EA		
c. Displacement greater than 1/2".  * * * {Severity H}	EA		
Defect:			
* Horizontal Joint Alignment:  Due to substructure or bearing settlement  Observation:			
<ul><li>a. Displacement less than 1/4".</li><li>***{Severity L}</li></ul>	EA		
b. Displacement greater than 1/4", less than 1/2".  * * * {Severity M}	EA		
c. Displacement greater than 1/2".  * * * {Severity H}	EA		

#### REFERENCES

- 1. U.S. Department of transportation, Federal Highway Administration, Bureau of Public roads (DOT) Bridge Inspector's Training Manual, 1990 Edition.
- 2. Bridge inspection and Rehabilitation, Parsons Brinckerhoff Edited by L.G. Silano, PE, 1993
- 3. Inspection of Bridges and Trestles NAVFAC MO-126, October 1991
- ACI Manual of Concrete Practice 1989, Part 1 Materials and General Properties of Concrete
- 5. AASHTO Manual for Maintenance Inspection of Bridges, American Association of State Highway and Transportation Officials
- 6. Micro Bridger (Version 1.0), U.S. Army Construction Engineering Research Laboratory

DOD CAS Manual		16 Bridge
	16.06 BRIDGE RAILING AND PARAPETS	
LEVEL II KEY	GUIDE SHEET CONTROL NUMBER	
N/A		
LEVEL III KEY	GUIDE SHEET CONTROL NUMBER	

#### LEVEL III GUIDE SHEET - KEY NO. 1

COMPONENT:

**TIMBER RAILING** 

CONTROL NUMBER: GS-III 16.06.01-1

#### Application

This guide has been prepared to identify the purpose of a Level III inspection and its more sophisticated test and inspection methods which may be appropriate to determine the cause and/or extent of defects recorded in Level I or Level II defect observations on a timber railing.

Whereas the purpose of the Level I inspection was to record the observable defects on the timber railing, this Level III inspection is performed to provide a thorough systematic evaluation of the observed defect and to make an assessment of it's effects, if left unchecked, on the safety, durability and stability of the railing and it's appurtenant works.

The Level III inspection should be performed when prompted by the results of a Level I or II inspection. The inspection should be performed by an engineer or multidisciplined team of engineers experienced in the design and construction of bridges.

The results of the Level III inspection will be used to develop maintenance or remedial measure work strategy that will correct the existing deficiency conditions or to require continued monitoring of existing deficiency conditions on the bridge railing.

In general, appropriate advanced inspection methods will be identified, recommended, and performed by or under the supervision of the inspection engineer personnel as part of the Level III test and inspection method. Advanced inspection methods will be assigned only after the assessment of defect conditions observed during a Level I or II inspection.

#### **Special Safety Requirements**

Passing traffic is a hazard. Level III inspection and testing must be performed with the prior approval of the Facility Manager who will notify the authorities to provide safety measures and safe access.

#### LEVEL III GUIDE SHEET - KEY NO. 1 (Continued)

COMPONENT: CONTROL NUMBER: TIMBER RAILING GS-III 16.06.01-1

#### Inspection Action

 Prior to making a field inspection of the observed defect, review all past records concerning the deck and the defective component if available. These records may include pre-construction investigation records, design criteria and analysis records, available construction records, previous periodic maintenance inspection records, water level records, and photographs taken during initial construction and subsequent inspections.

- 2. Perform inspection of the pertinent components where observed defects that triggered a Level III inspection are listed.
- Make an assessment of the importance of individual defects observed for a given component at the bridge site. Indicate priorities for any required maintenance, or remedial measure work.
- 4. Identify whether particular observed defects need additional or continued observation.
- 5. Assess the stability and safety of the bridge railing.
- 6. Prepare final cost estimate for advanced inspection methods required to determine the cause and extent of the observed defect.
- 7. Prepare cost estimate for required maintenance or remedial repair measures, as applicable.

Level III advanced inspection methods may be required for specific Level I and Level II defect conditions observed at a bridge site. Level III advanced test or inspection methods and associated observed defects for bridge railing include, but are not limited to the following:

#### LEVEL III GUIDE SHEET - KEY NO. 1 (Continued)

COMPONENT:

TIMBER RAILING

**CONTROL NUMBER:** 

GS-III 16.06.01-1

#### 

1. Increment borer

interior and exterior deterioration of timber due to

decay or parasites

2. Ultrasonic

interior deterioration

3. Moisture content

deterioration due to decay or parasites

4. Survey measurements

railing movement

#### **Special Instructions**

Review as-built and design drawings of structure.

### **Special Tools & Equipment Requirements**

Increment borer Surveying equipment Moisture meter

# **Recommended Inspection Frequency**

As triggered by Level I and II defect/observations or every 3 years.

#### References

- 1. U.S. Department of transportation, Federal Highway Administration, Bureau of Public roads (DOT) Bridge Inspector's Training Manual, 1990 Edition
- 2. Bridge inspection and Rehabilitation, Parsons Brinckerhoff Edited by L.G. Silano, PE, 1993
- 3. Inspection of Bridges and Trestles NAVFAC MO-126, October 1991
- 4. AASHTO Manual for Maintenance Inspection of Bridges, American Association of State Highway and Transportation Officials
- 5. Micro Bridger (Version 1.0), U.S. Army Construction Engineering Research Laboratory

#### **DESCRIPTION**

The purpose of a drainage system is to remove water and all hazards associated with it from the structure.

# SPECIAL TOOL AND EQUIPMENT REQUIREMENTS

The following list of special tools and equipment, beyond the requirements listed in the Standard Tool Section, may be required to perform the inspection of this subsystem.

- 1. Boat
- 2. Related safety equipment

### SPECIAL SAFETY REQUIREMENTS

Level I and Level II inspections are performed by walking the bridge deck and observing from ground level, thus passing traffic is a hazard. The inspection must be performed with the prior arrangement and approval of the Facility Manager who will notify the necessary authorities so as to provide traffic safety measures and access. Inspector will be required to wear orange safety vest.

Depending on the bridge type, a boat may be required to observe certain components. Inspector will take all necessary safety measures and refer to the Master Safety Plan for guidance and compliance.

#### COMPONENT LIST

- ◆ 16.07.01 DECK DRAIN
- ◆ 16.07.02 OUTLET PIPES
- ◆ 16.07.03 DOWNSPOUT PIPES
- ◆ 16.07.04 DRAINAGE TROUGH

#### **RELATED SUBSYSTEMS**

Due to the related nature of the elements requiring inspection, the following should be reviewed for concurrent inspection activities.

19.01	ROADWAYS
20.01	RAILROAD
21.00	WATERFRONT (all subsystems)
29.00	SITE ELECTRICAL (all subsystems)

### STANDARD INSPECTION PROCEDURE

The Level I inspection shall be carried out for each component listed. The inspector will identify any physical defects existing in each component; will note the level of severity of each defect type; and measure the quantity of each defect.

#### **COMPONENTS**

#### ◆ 16.07.01 DECK DRAIN

The deck drain is a receptacle to receive water and remove it from the deck area. Deck drains may be nothing more than openings in a filled grid deck, holes in concrete deck, or slots in the base of a parapet. Inlet boxes and scuppers are examples of deck drains. Debris is the principal cause of drain inlet clogging. The ponds, or puddles of water, that form on the bridge deck pose the problem of hydroplaning and icing which can cause accidental damage and other extensive bridge deterioration.

Defect:	UOM	LEVEL II KEY	KEY
* Clogging of Inlets:			
Observation:			
<ul> <li>a. Inlet partially clogged.</li> </ul>	EA		
* * * {Severity L}			
b. Inlet totally clogged.	EA		
* * * {Severity M}	05		
<ul> <li>c. Soil or sand accumulation on deck at inlet.</li> </ul>	SF		
* * * {Severity M}			
Defect:			
* Deck Drain Grating:			
Observation:			
a. Cracked grate.	EA		
* * * {Severity L}			
b. Broken grate.	EA		
* * * {Severity H}	<b>5</b> A		
c. Missing grate.	EA		
* * * {Severity H}			

# **COMPONENTS (Continued)**

#### **◆ 16.07.02 OUTLET PIPES**

The outlet pipe conducts water away from the deck drain. For bridges over roadways, the outlet pipe connects to a downspout pipe system. When the bridge is not over a roadway, the outlet may simply extend a few feet down from the deck so that drainage is not windblown onto the superstructure.

Defect:	UOM	LEVEL II KEY	LEVEL III KEY
* Deck Outlet Discharge:			
Observation:  a. Water stain and/or deterioration of members near outlet discharge.  * * * {Severity M}	EA		
b. Water discharge on roadway below. ***{Severity M}	EA		
<ul><li>c. Ground erosion below deck outlet.</li><li>***{Severity H}</li></ul>	EA		
Defect:			
Pipe Corrosion:			
Observation:  a. Surface corrosion no pitting evident.  * * * { Soverity   }	LF		
<ul><li>* * * {Severity L}</li><li>b. Corrosion evidenced by pitting no blistering.</li></ul>	LF		
<pre>* * * {Severity M} c. Corrosion evidenced by holes or     loss of base metal. * * * {Severity H}</pre>	LF		

COMPONENTS (Continued)			
◆ 16.07.02 OUTLET PIPES (Continued)		LEVEL II	LEVEL III
Defect:	UOM	KEY	KEY
* Damage Outlet Pipe:  Deck outlet can be damaged from freezing and collision Observation:			
a. Pipe joint leaking.  * * * {Severity L}	EA		
b. Crack in pipe with visible water leakage.	EA		
***{Severity M} c. Broken pipe. ***{Severity H}	EA		
Defect:			
* Supports and Fasteners: Observation: a. Loose supports or fasteners.	EA		
<pre>***{Severity L} b. Missing supports or fasteners. ***{Severity H}</pre>	EA		
Defect:			
* Connectors: Observation: a. Outlet pipe connection leaks at deck drain.	EA		
<ul><li>* * * {Severity L}</li><li>b. Outlet pipe disconnected at deck drain.</li></ul>	EA		•
<pre>***{Severity M} c. Outlet pipe connector missing</pre>	EA		

# **COMPONENTS (Continued)**

# **◆ 16.07.03 DOWNSPOUT PIPES**

When a bridge is located over a roadway, the deck drainage must be directed from the outlet pipe to a nearby storm sewer system or another appropriate release point. This is done with a downspout pipe network. Downspouts and horizontal runs which have inadequate slope and sharp directional changes are subject to the plugging of drains.

Defect:	UOM	LEVEL II KEY	LEVEL III KEY
* Downspout Discharge:			
Observation:			
<ul> <li>a. Water stain and/or deterioration of members near downspout discharge.</li> </ul>	EA		
* * * {Severity M}			
<ul> <li>b. Water discharge from downspout on roadway below.</li> </ul>	EA		
* * * {Severity M}			
<ul> <li>Ground erosion below downspout outlet.</li> </ul>	EA		
* * * {Severity H}			
Defect:			
Pipe Corrosion:			
Observation:			
<ul><li>a. Surface corrosion no pitting evident.</li><li>***{Severity L}</li></ul>	LF		
b. Corrosion evidenced by pitting no blistering.	LF		
* * * {Severity M}			
c. Corrosion evidenced by holes or	LF		
loss of base metal.	<b>L</b> .		
* * * {Severity H}			
(0010111)			

# **COMPONENTS (Continued)**

# ◆ 16.07.03 DOWNSPOUT PIPES (Continued)

Defect:	иом	LEVEL II KEY	LEVEL III KEY
* Pipe Slope:			
Observation:			
<ul><li>a. Pipe slope less than 3/16" per foot, greater than 1/8" per foot ***{Severity L}</li></ul>	LF		
b. Pipe slope less than 1/8" per foot greater than 1/16" per foot	LF		
<pre>***{Severity M} c. Pipe slope less than 1/16" per foot</pre>	LF		
* * * {Severity H}			
Defect:			
* Damaged Pipe:			
Downspout pipe can be damaged from freezing and			
exterior forces.			
Observation:	EA		
a. Pipe joint leaking.	LA		
* * * {Severity L} b. Crack in pipe with visible	LF		
water leakage.			
* * * {Severity M}			
c. Broken pipe.	LF		
* * * {Severity H}			
Defect:			
* Supports and Fasteners:			
Observation:			
a. Loose supports or fasteners.	EA		
* * * {Severity L}			
<ul><li>b. Missing supports or fasteners.</li><li>***{Severity H}</li></ul>	EA		

# **COMPONENTS (Continued)**

# ◆ 16.07.04 DRAINAGE TROUGH

Drainage troughs are located under expansion joints to carry away the deck runoff water which passes through the joint.

Defect:	иом	KEY	LEVEL III KEY
* Trough:			
Observation:			
<ul><li>a. Trough shows signs of leaking.</li><li>***{Severity L}</li></ul>	LF		
<pre>b. Trough full of rocks and debris. ***{Severity L}</pre>	LF		
<ul><li>c. Trough is cracked or torn.</li><li>***{Severity H}</li></ul>	LF		
<pre>d. Trough discharging onto structure. ***{Severity H}</pre>	LF		
<pre>e. Trough missing. ***{Severity H}</pre>	LF		
Defect:			
* Trough Slope: Observation:			
<ul><li>a. Trough slope less than 3/16" per foot greater than 1/8" per foot.</li><li>* * * {Severity L}</li></ul>	LF		
<ul><li>b. Trough slope less than 1/8" per foot greater than 1/16" per foot.</li><li>***{Severity M}</li></ul>	LF		
<ul><li>c. Trough slope less than 1/16" per foot.</li><li>***{Severity H}</li></ul>	LF		
Defect:			
* Supports and Fasteners: Observation:			
<ul><li>a. Loose supports or fasteners.</li><li>***{Severity L}</li></ul>	EA		
<ul><li>b. Missing supports or fasteners.</li><li>***{Severity H}</li></ul>	EA		

# **REFERENCES**

- 1. U.S. Department of transportation, Federal Highway Administration, Bureau of Public roads (DOT) Bridge Inspector's Training Manual, 1990 Edition.
- 2. Bridge inspection and Rehabilitation, Parsons Brinckerhoff Edited by L.G. Silano, PE, 1993
- 3. Inspection of Bridges and Trestles NAVFAC MO-126, October 1991
- 4. ACI Manual of Concrete Practice 1989, Part 1 Materials and General Properties of Concrete
- 5. AASHTO Manual for Maintenance Inspection of Bridges, American Association of State Highway and Transportation Officials
- 6. Micro Bridger (Version 1.0), U.S. Army Construction Engineering Research Laboratory

DOD CAS Manual		16 Bridge
	16.07 DRAINAGE	
LEVEL II KEY	GUIDE SHEET CONTROL NUMBER	
N/A		
LEVEL III KEY	GUIDE SHEET CONTROL NUMBER	

N/A

# **APPENDIX A**

#### **ABBREVIATIONS**

AASTHO American Association of State Highway and Transportation Officials

ABUT Abutment

ACI American Concrete Institute

AGO Associated General Contractors

ARTBA American Road and Transportation Builders Association

BRG Bearing

CONC Concrete

DIA Diameter

EA Each

EFFLOR Efflorescence

FT Feet

H High

IN Inches

INFILT Infiltration

L Low

LF Linear Feet

LS Low Severity

M or MED Medium

MS Medium Severity

PCS Pieces

SEVRL Several

SF Square Feet

UOM Unit of Measure

W/ With

# APPENDIX A

WKS Weeks

WT Water

YR Year

< Less Than

> Greater Than

% Percent

/ And

#### Glossary

Abutment

A substructure composed of stone, concrete, brick, or timber supporting the end of a single span or the extreme end of a multispan superstructure and, in general, retaining or supporting the approach embankment placed in contact therewith.

Cantilever Abutment

An abutment in which the stem or breast wall is fixed rigidly to the footing. The stem, acting as a cantilever beam transmits the horizontal earth pressure to the footing, which maintains stability by virtue of the dead weight of the abutment and of the soil mass resting on the rear portion, or heel, of the footing.

Gravity Abutment

A heavy abutment with which resist the horizontal earth pressure by its own dead weight.

Aggregate

Inert minerals such as sand, gravel, and crushed stone. The aggregates are divided into two sizes, fine and coarse.

Alligator or Fatigue Cracking Series of interconnecting cracks caused by failure of the asphalt concrete surface under repeated traffic loading. The cracking initiates at the bottom of the asphalt surface where tensile stress and strain is highest under a wheel load. The cracks propagates to the surface initially as one or more longitudinal parallel cracks. After repeated traffic loading, the cracks connect, forming many-sided, sharp-angled pieces that develop a pattern resembling chicken wire or the skin od an alligator. The pieces are usually less than one foot on the longest side. Alligator cracking occurs only in areas that are subjected to repeated traffic loadings. Therefore, it would not occur over an area unless the entire area was subjected to traffic loading. Alligator cracking does not occur in asphalt overlays on concrete slabs. Alligator cracking is considered a structural distress.

Anchor Bolt

A shaft-like piece of metal commonly threaded and fitted with a nut and washer at one end only, used to secure in a fixed position upon the substructure the bearing of a bridge, the base of a column, a pedestal, shoe or other member of a structure.

Asphalt Bleeding

A film of bituminous material on the pavement surface which creates a shiny, glass-like, reflecting surface that usually become quite sticky. Bleeding is cause by excessive amount of asphalt cement in the mix and/or low air void content. It occurs when asphalt fills the voids of the mix during hot weather and then expands out onto the surface of the pavement. Since the bleeding process is not reversible during cold weather, asphalt will accumulate on the surface.

Approach slab

A reinforced concrete slab placed on the approach embankment to and usually resting upon the abutment back wall. The function of the approach slab is to carry wheel loads on the approaches directly to

the abutment, thereby eliminating any approach roadway misalignment due to approach embankment settlement.

Armor A secondary steel member installed to protect a vulnerable part of

another member, steel angle placed over the edge of a joint.

Backfill Material placed adjacent to an abutment, pier, retaining wall or other

structure or part of a structure to fill the unoccupied portion of the foundation excavation. Soil, usually granular, placed behind and

within the abutment and wingwalls.

Backwall The topmost portion of a abutment above the elevation of the bridge

seat, functioning primarily as a retaining wall with a live load surcharge. It may serve also as a support for the extreme end of the

bridge deck and the approach slab.

Bearing A support element transferring loads from superstructure to

substructure while permitting movement capability.

Bearing seat A prepared horizontal surface at or near the top of a substructure

unit upon which the bearing are placed.

Bolt A mechanical fastener with machine threads at one to receive a nut,

and a hexagonal head at the other end.

Breastwall The portion of an abutment between the wings and beneath the

bridge seat; the breast wall supports the superstructure loads, and

retains the approach fill.

Bridge A structure spanning and providing passage over river, chasm, road,

or the like.

Cantilever A structural member which has a free end projecting beyond its

supporting wall or column, length of span overhanging the support.

Cast-in-place The act of placing and curing concrete within formwork to construct

a concrete element in its final position.

Cement A powder that hardens when mixed with water, an ingredient used

in concrete.

Chord A generally horizontal member of a truss.

Concrete A mixture of aggregate, water, and a binder, usually portland

cement, which hardens to a stone like mass.

Corrosion

The general disintegration and wasting of surface metal or other material through oxidation, decomposition, temperature, and other natural agencies.

Cracking

A crack is a linear fracture in concrete. Cracks may extend partially longitudinal) or completely through the concrete member. On concrete decks and beams, the two basic types of cracks are structural and nonstructural cracks. Structural cracks are caused by dead loads and live loads stresses. Nonstructural cracks are divided into three categories temperature, The wearing surface provides the riding surface for

Curbs

A barrier paralleling the side limit of the roadway to guide the movement of vehicle wheels and safeguard bridge trusses, railing or other constructions existing outside the roadway limit and also pedestrian traffic upon sidewalk from collision with vehicles and their loads.

Dead load

A static load due the weight of the structure itself.

Deck (Structural)

The bridge deck is to provide a roadway over which traffic can move and to distribute traffic and deck weight loads to the superstructure.

Deflection

Elastic movement of a structural member under a load.

Delamination

Delamination occurs when a layer of concrete separates form the bridge deck or beam at or near the level of the outermost layer of reinforcing.

Diaphragm

A member placed within a member or superstructure system to distribute stresses and improve strength and rigidity.

Diver

A specially trained individual who inspects the underwater portion of a bridge substructure and the surrounding channel.

Drainage

A system design to remove water from a structure.

Dye Penetrant

A dye penetrant can be used to define the extent and size of surface flaws in steel members.

Edge Cracking

Cracks parallel to and usually within one to two feet of the outer edge of the pavement. Distress is accelerated by traffic loading and can be caused by frost-weakened base or subgrade near the edge of the pavement. The area between the crack and pavement edge is classified as raveled if it breaks up (sometimes to the extent that pieces are removed).

Efflorescence

Efflorescence is a white deposit on the concrete caused by crystallization of soluble salts (calcium chloride) brought to the surface by moisture in the concrete. Efflorescence is caused by

moisture absorption and flow. It is a indication that the concrete is contaminated.

Elastomer

A natural or synthetic rubber-like material.

Erosion

Wearing away of soil by flowing water.

**Expansion Joint** 

A joint designed to provide means for expansion and contraction movement provided by temperature changes, loading or other agencies.

Flange

The horizontal parts of a rolled I-shape beam or of a built-up girder extending transversely across the top and bottom of the web.

Floor system

The complete framework of members supporting the bridge floor and the traffic loading

Footing

The enlarged, lower portion of a substructure, which distributes the structure load with to the earth or to supporting piles. the most common footing is the concrete slab.

Girder

A flexural member which is the main or primary for the support for the structure, and which usually receives loads from floor beam and stringer, any large beam especially if built up.

**Ground-Penetrating** 

This technique uses low-power, high frequency pulsed radar. And important benefit of this method is the ability to measure the thickness of a material.

Guardrail

A safety feature element intended to redirect an errant vehicle away from the approach embankment.

Half Cell Test

This test measures the tendency for corrosion in embedded reinforcing as a display of the electrical potential between two points in steel.

Hairline cracks

Very small cracks that form in the surface of concrete due to tension caused by loading.

Honeycombs

Honeycombs are hollow spaces or voids that may be present within the concrete. Honeycombs are caused by improper vibration during construction, resulting in the segregation of the coarse aggregates from the fine aggregate and cement paste.

Hydroplaning

Loss of contact between a tire and the deck surface when the tire planes or glides on al film of water covering the deck.

Infrared Thermographic This Technique uses and infrared camera to detect temperature

differentials in a concrete surface. A "cold spot" indicates a

delamination.

Inlet An opening in the floor of a bridge leading to a drain.

Joint In stone masonry, the space between individual stones; in concrete,

a division in continuity of the concrete; in a truss, point at which

member of a truss frame are joined.

Longitudinal Bracing The bracing assemblage engaging the columns of trestle and viaduct

bents and towers in perpendicular or slightly inclined planes located lengthwise with the bridge structure and functioning to resist the longitudinal forces resulting from traffic traction and momentum, wind or other forces tending to produce longitudinal movement and

deformation.

Magnetic Particle This test is useful in detecting surface gouges, cracks, and holes. A

magnetic field is induced into a member, and cracks or other irregularities in the surface of the member causes irregularities in the

magnetic field.

Masonry A general term applying to abutment, piers, retaining walls, arches

and allied structures built of stone, brick or concrete and known

correspondingly as stone or concrete masonry.

Member An individual angle, beam, plate forgiving, casting or built piece,

with or without connected parts for joints, intended ultimately to

become an integral part of an assembled frame or structure.

Mortar An intimate mixture, in a plastic condition, of cement, or other

cementitious material with fine aggregate and water, used to bed and bind together the quarried stones, bricks or other solid materials composing the major portion of a masonry construction or to

produce a plastic coating upon such constructions.

Moisture Content Moisture meter can be used to determine moisture content in a

timber member. Moisture contents exceeding 20% indicate the

condition of the wood is conducive to decay.

Outlet In Hydraulics, the discharge of a drain, sewers, or culverts.

Out-of-Alignment Bowing, deflection, or other movement that brings the member out

of plumb or not level in one or more direction.

Patch Deterioration Deterioration of an area where the original pavement has been

removed and replaced with either similar or different material.

**Parasites** 

A plant or animal that lies on or within another from which it derives sustenance.

Parapet

A wall-like member composed of brick, stone or reinforced concrete construction upon the retaining portion of an approach cut, embankment or causeway or along the outermost edge of the roadway or the sidewalk portion of a bridge to serve as a protection to vehicular and or pedestrian traffic.

Polished Aggregate

It is caused by repeated traffic applications. when the aggregate in the surface become smooth to the touch, adhesion with vehicle tires is considerably reduced. When the portion of aggregate extending above the surface is small, the pavement texture does not significantly contribute to reducing vehicle speed. Polished aggregate should be counted when close examination reveals that the aggregate extending above concrete is negligible, and the surface aggregate is smooth to the touch. This type of distress is indicated when the number on a skid resistance test is low or has dropped significantly from previous ratings.

Pile Pier or Bent

A pier composed of driven piles capped or decked with a timber grillage or with a reinforced concrete slab forming the bridge seat.

Pile Cap

The top most portion of a pier. On rigid frame piers, the term applies to the beam across the column tops. On hammerhead and tee piers, the cap is a continuous beam.

Pile

A rod or shaft-like linear member of timber, steel, concrete, or composite materials driven into the earth to carry structure loads thru weak strata of soil to those strata capable of supporting such loads. Pile are also used where loss of support due to scour is expected.

Pile Cap

Concrete footing for pier or abutment supported on plies. Also applied to the concrete below the pile tops when footing reinforcing steel is placed completely above the piles.

Pier

A structure composed of stone, concrete, brick, steel or wood and built in a shaft or blocklike form to support the ends of the spans of a multi-span superstructure at intermediate location between its abutments.

Pitting

Development of relatively small cavities in a surface; in concrete, localized disintegration, such as a popout; in steel, localized corrosion evident as minute cavities on the surface.

Plate Girder An I-shaped beam composed of a solid plate web with either flange

plates or flange angles bolted, riveted or welded upon its edges. Addition cover plates may be attached to the flanges to provided

greater flange area.

Probing Probing consists of inserting a pointed tool, such as an ice pick, into

the wood and comparing its resistance with that of sound wood.

Portal The clear unobstructed space of a through bridge forming the

entrance to the structure.

Pot Holes Small worm or disintegrated areas of floor or approach surface

concaved by the wearing action of vehicle wheels.

Pop-out Conical fragment broken out of concrete surface. Normally about

one inch in diameter. Shattered aggregate particles usually found at

bottom of hole.

Primary members The main load caring members in the superstructure.

Railing A wooden, concrete or metal fence-like construction built at the side

of the roadway, or the sidewalk, upon the retaining wall portion of an approach cut, edge of the roadway or sidewalk portion of a bridge to guard or guide the movement of both pedestrian and vehicular traffic and to prevent the accidental passage of traffic over

the side of the structure.

slab beneath the asphalt concrete surface because of thermal and moisture changes; it is generally not load-initiated. However, traffic loading may cause a breakdown of the (AC) near the initial crack, resulting in spalling. A knowledge of slab dimensions beneath the

(AC) surface will help to identify these cracks.

Reinforced concrete 
Concrete with steel reinforcing bars bonded with it to supply

increased tensile strength and durability.

Reinforcing Bar (Rebar) A steel bar, plain or with a deformation surface, which bonds to the

concrete and supplies tensile strength to the concrete.

Rutting A surface depression in the wheel paths. Pavement uplift may occur

along the side of the rut; however, in many instances, ruts are noticeable only after a rainfall, when the wheel paths are filled with water. Rutting stem from a permanent deformation in any of the pavement layers or subgrade, usually by consolidation or lateral movement of the materials due to traffic loads. Rutting may be

caused by plastic movement in the mix in hot weather or inadequate

compaction during construction. Significant rutting can lead to major structural failure of the pavement and hydroplaning potential. Wear of the surface in the wheel paths from studded tires can also cause a type of "rutting".

Scaling

Scaling is the gradual and continuing loss of surface mortar and the aggregate over and area. Scaling is classified as light, medium or severe.

Scour

Erosion of a river bed area caused by stream flow.

Scupper

An opening in the floor portion of a bridge to provide means for rain or other water accumulated upon the roadway surface to drain through it into the space beneath the structure.

Secondary Member

A member that is carried by other members and does not resist traffic load. The function of a secondary member is to brace and stiffen the primary members.

Section loss

Loss of a member cross sectional area usually by corrosion or decay.

Sidewalks

The portion of the bridge floor area serving pedestrian traffic only and, for safety and convenience to its users, commonly elevated above the portion occupied vehicles.

Sole plate

A plate attached to the bottom flange of a beam that distributes the reaction of the bearing to the beam.

Sounding

Determining the depth of water by an echo-sounder or sounding line.

**Spalling** 

A spall is a roughly circular or oval depression in the concrete. Spall results from the separation and removal of a portion of the surface concrete, revealing a fracture roughly parallel to the surface. Spall can caused by corroding reinforcement and friction form thermal movement.

Stiffener

A small member attached to another member to transfer stress and to prevent buckling.

Truss

A jointed structure made up of individual member arranged and connected usually in a triangular pattern, so as to support longer spans.

Warping

A deviation of a member or surface from its original shape, usually caused by either temperature or moisture differential or both within the member.

Web

The portion of a beam located between and connected to the flanges.

Wearing Surface The wearing surface provide the riding surface for traffic and is

placed on top of the structural deck.

The affects caused by light, water and heat. Weathering can change Weathering

the equilibrium moisture content in the wood, thereby resulting in a

change.

Weld A joint between pieces of metal at faces which have been made

plastic by heat or pressure.

Wheel loads The load carried by and transmitted to the supporting structure by

one wheel of a traffic vehicle.

The retaining wall extension of an abutment intending to restrain and Wingwall

hold in place the side slope material of an approach roadway

embankment.

Ultrasonic Test Ultrasonic testing consists of high frequency sound waves

introduced by a sending transducer. Discontinuities in the specimen interrupt the sound wave and deflect it toward a receiving

transducer.

# **APPENDIX C**

#### LIFE CYCLES

#### 16 BRIDGES

#### 16.01 ABUTMENTS

**Abutments** 

**50 YRS** 

Source:

AASHTO-AGO-ARTBA Joint Committee Corrosion Protection of Concrete Components in Bridges November, 1992

# 16.02 PILES

Piles

**50 YRS** 

Source:

AASHTO-AGO-ARTBA Joint Committee Corrosion Protection of Concrete Components in Bridges November, 1992

#### 16.03 PIERS

Pier

**50 YRS** 

Source:

AASHTO-AGO-ARTBA Joint Committee Corrosion Protection of Concrete Components in Bridges November, 1992

# 16.04 SUPERSTRUCTURE

Superstructure

**50 YRS** 

Source:

AASHTO-AGO-ARTBA Joint Committee Corrosion Protection of Concrete Components in Bridges November, 1992

# **APPENDIX C**

# LIFE CYCLES

# 16 BRIDGES (Cont.)

# 16.05 BRIDGE DECKS

Timber Decks	20 YRS
Concrete Decks	20 YRS
Steel Decks	20 YRS

### Source:

Road and Bridge Magazine December 1993

European PCC demo draws AASHTO engineers by Tom Kuennen

#### 16.06 BRIDGE RAILING AND PARAPETS

Timber Railing	20 YRS
Concrete Railing	20 YRS
Steel Railing	20 YRS
Parapet	20 YRS

Source:

Road and Bridge Magazine December 1993

# 16.07 DRAINAGE

Drainage Device 20 YRS

Source:

Road and Bridge Magazine December 1993